



**ADDENDUM NO. 1**

**PROJECT TITLE:** GA Apron and Taxiway C

**DATE:** June 2, 2025

This addendum shall be included in, and be considered a part of the plans and specification for the above named project.

Addendum No. 1 is issued to notify you of the revisions and/or corrections to the following items that were posted on the City of Coolidge website (coolidgeaz.com/bidpackage).

**BEGIN ADDENDUM NO. 1**

Prospective bidders and interested parties are hereby directed to make the following changes to the plans and specifications for the above referenced project. This addendum shall be affixed to the contract documents and the signature of the receiving party shall be placed on page ‘B – 1’ of the specifications.

**I. SPECS DIVISION I – Bid Proposal**

Remove Pages B-1 through B-37 and replaced with attached “*Specifications Division I – Bid Proposal\_Addendum 01.pdf*” including the following updates:

**a. Bid Proposal, pages B-3 – B-7, Bid Schedule:**

Updated Project Quantities for Base Bid, Alternate 1, and Alternate 2. Updated item numbers have been highlighted.

**b. Bid Proposal, pages B-8, B-13, B-15, B-23, Date of Submittal:**

Updated all dates of signature to read “202\_\_”

**II. SPECS DIVISION IV – Item M-002 Engineer’s Field Office and Curing Facilities:**

This specification can be omitted from the required documents as a Field Office and On-site Curing Facility is not a requirement for this project.

**III. SPECS DIVISION V – Technical Specifications**

The following FAA technical specifications have been included and shall be utilized for the specified material acceptance.

- a. P-209 Crushed Aggregate Base Course

**IV. PLAN SHEETS:**

**Remove** the following plan sheets and **Replace** with the attached sheets:

Sheet Number	Title Description	Narrative
C1.2	Notes	Added sheet E2.6 to Sheet List



C1.3	Summary of Quantities	Updated project quantities for Base Bid, Alt 1, and Alt 2
C1.4	Project Layout	Updated Primary Control Point
C3.1	Typical Sections	Updated Subbase to P-152
C3.2	Typical Sections	Updated Subbase to P-152
C3.3	Typical Sections	Updated Subbase to P-152
C3.4	Typical Sections	Updated Subbase to P-152
C5.3	Storm Drain Details	Updated Base Course Spec Reference
E1.0	Electrical Scope of Work	Updated Alt 1 map
E1.1	Electrical Layout Plan	Updated Handhole, Sign, and Light Tables
E1.2	Electrical Layout Plan	Updated Handhole, Sign, and Light Tables
E3.2	Electrical Circuit Map	Updated Circuit Map
E2.6	Electrical Details	Added sheet and detail

**V. Geotechnical Report**

The attached Geotechnical Report shall become a part of the above referenced project contract documents.

**VI. Pre-Bid Attendance Sheet**

The attached attendance sheet shall become a part of the above referenced project contract documents.

**VII. Pre-Bid Meeting Questions**

The following questions were generated as a result of the Pre-Bid Meeting held on May 28, 2025 at 10:00 AM.

**Q01** Article 15 specifies that bidders must submit the entire contract document book. The file P08\_GA\_Apron\_Twy\_C\_IFB\_Specifications.pdf appears to contain 407 pages. Could you please confirm whether bidders are required to submit all 407 pages with their bid, in addition to the required forms? If not, could you kindly specify which pages must be submitted in order for a bid to be considered responsive?

**A01** B-1 through B-37 (Bid Proposal) and Attachment A (Subcontracting and Good Faith Efforts Summary), Attachment B (Letter of Intent to Perform as a Subcontractor/Subconsultant/Supplier), Attachment C (Identification Statement for Disadvantaged Business Enterprises), including items A – N in Article 15 of Division I Instructions to Bidders are the only required submittal documents.

**Q02** Can you confirm the Handholes are to be H-20 Rated – construction note mentions Aircraft rated.

**A02** Confirmed, handholes are to be H-20 rated

**Q03** Can you clarify the Directional drilling bid items for Base Bid and Alternate. Does the base bid require 2-2” and the Alternate require an additional 2-2” at the same location? Bid schedule shows 4-2” for each bid item.

**A03** The bore will be in the one location base bid only and it will be 4-2”



**Q04** Can you confirm the DBE goal for this project as Section A2 lists a DBE goal of 1.90%, however other portions of the specifications state that there is not a DBE goal and we're only required to document Good Faith Efforts.

**A04** DBE goal for this project is 1.90%

**Q05** Are you able to provide the geotechnical report?

**A05** Geotechnical report will be made available in Addendum 01

**Q06** Can you confirm the RipRap quantity as the measured plan quantities do not align with the Base Bid and Alt. 1 bid quantities?

**A06** Base bid RipRap quantity should equal 150 SY. Alternate 1 quantity should equal 50 SY

**Q07** Can you confirm the aggregate bedding specification as detail 1 on sheet C5.3 notes a MAG and FAA Spec?

**A07** Bedding should be per MAG 701. Note on C5.3 has been updated and the sheet is included in this addendum

**Q08** For project cost savings would it be feasible to have QC testing done at offsite facilities instead of having an on-site QC lab?

**A08** No objections to performing QC testing at an offsite facility

**Q09** Can Phase 1 and Phase 2 be constructed concurrently, or must phase 1 be completed prior to Phase 2 commencement?

**A09** Phase 1 and Phase 2 can be constructed concurrently. Airport will need to close the primary Runway 5-23 during Phase 2 so advanced notice is required.

**Q10** How many days are allocated for each phase within the base bid, alt. 1, and alt. 2?

**A10** 55 days for Base Bid; 20 days for Alt 1; 15 days for Alt 2

**Q11** Is the on-site stockpile shown on the plans a long or short-term stockpile? Will excess materials become property of the airport?

**A11** The stockpile shown on the plans is a long-term stockpile. All excess materials will become property of the airport.

**Q12** Is there an available on-site water source?

**A12** There are available fire hydrants that can be tapped with a meter

**Q13** Are the chains and hardware incidental to the aircraft tie-down bid item?

**A13** Yes, the chains and hardware are incidental to the aircraft tie-down bid items.

**Q14** The electrical items listed in Alternative 1 Bid appear to include the quantities also listed in the Base Bid instead of the additional quantities for the Alternative 1 Bid; what are the correct electrical quantities for Alternative 1 Bid?

**A14** The corrected electrical quantities have been included in Addendum 1

**Q15** At the pre-bid meeting it was mentioned there is a water hydrant located near the hangars. Is the contractor responsible for paying any fees for use of this hydrant or is there no charge to the contractor?

**A15** The contractor is responsible for paying any fees imposed by the City of Coolidge for use of the hydrant

**Q16** Please confirm the onsite stockpiled material located at the southwest corner of Parachute Dr. and Beachcraft Rd. is suitable for the project fill required?



- A16** Confirmed, the stockpiled material at the referenced location is an acceptable source of fill for the project and will be required to be excavated, placed, and compacted per FAA specification P-152 Excavation, Subgrade, and Embankment.
- 
- Q17** What are the restrictions on runway closures? What is the max duration that the runway may be closed in the base bid, alt. 1, and alt. 2?
- A17** The runway will be closed only while active construction activities are taking place within the Runway Object Free area.
- 
- Q18** Is milling required for asphalt removal?
- A18** Milling of the asphalt shoulder pavement is not required for removal.
- 
- Q19** Due to the short duration of the project will the engineers field office and curing facility be required? If not – will this item be removed?
- A19** Engineers field office and curing facility are not required. This specification can be omitted from the book and bid item has been removed.
- 
- Q20** In the Base Bid and Bid Alternate #1 the taxiway edge lights use a single-piece base can due to crushed aggregate shoulders. In Bid Alternate #2, with paved asphalt shoulders, typically a 2-piece adjustable base can and asphalt coring are utilized in place of the single-piece can. Where should the additional cost for the 2-piece base can and coring be accounted for in Bid alternate 2?
- A20** Added sheet E2.6 to Alternate 2 includes detail for in-pavement edge light. Added line item to Alternate 2 for associated cost.
- 
- Q21** Can you please confirm the following Bid Quantities: Unclassified Excavation, Fill with Select Material, Aggregate Base Course (Stabilized Shoulder)
- A21** Bid quantities should reflect what has been listed within Addendum 01
- 
- Q22** We are not able to quote the exact mix the attached specs call for, as some of the requirements we do not make in Coolidge, Arizona. My question to you is if you believe that they will make an exception to our 4500 PSI .45 W/C mix using Coolidge materials and non air entrained mixes? We have poured at this airport several times over the years.
- A22** Concrete mix designs specified for light can bases and sign bases should conform to the specified P-610 specification. If there is deviation with the submitted mix design, MAG 725 Class AA concrete will be utilized as a secondary option for compliance and approval.
- 
- Q23** Please provide a detail for the (4) catch basin dimensions.
- A23** Catch basins are to be constructed per MAG standard details 537 Type G. Rim and Invert elevations are per plans
- 
- Q24** There doesn't seem to be an outlet for the storm drain system involving the (2) culverts. Where does the water go?
- A24** Downstream catch basin will serve as a bubble-up structure.
- 
- Q25** On sheet 4 of the plans the elevation for the primary control point is given as 157.99; that elevation seems incorrect as it is 1400-feet below the elevation of control point number 2. Please confirm.
- A25** The correct control point elevation is 1575.99
- 
- Q26** The drainage channel north of Taxiway C1 and the Apron seems to have a flat bottom at STA 103, but then it's a V ditch at STA 102 and is a flat bottom at STA 101. Could you please provide a cross sectional detail of the drainage channel?
- A26** Drainage channel is to commence at the Runway Object Free Area and transition from a low-point flow line to a flat bottom 10' wide channel with varying side slopes until ultimate outfall as shown per plan.
- 
- Q27** From the details shown on sheet 14 of the plans, it appears that after a rain event the catch basin on the left side of Taxiway C3 will be filled with water to the level of 1571.34 and the catch basin on the right will have standing water at the same level of 1571.34, which is only 4/10 below the rim. This standing water



Coolidge Municipal Airport  
GA Apron and Taxiway C  
FAA AIP 3-04-0011-018-2025 (Design)  
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Addendum No. 1

will remain, except for evaporation and seepage from the joints at the bottom of the culvert trench. Am I correct in the interpretation of the water flow after a rainstorm?

A27 This is the correct interpretation of the water flow after a rain event.

**End Addendum No. 1**



# **ATTACHMENT I**

## **SPECS DIVISION I - BID PROPOSAL**



**BID PROPOSAL**

(Exhibit A of the Construction Contract)

This Bid Proposal shall not be detached from the Contract Documents. The entire Specification Book shall be returned with the executed Bid.

**PROJECT IDENTIFICATION:**

**COOLIDGE MUNICIPAL AIRPORT  
COOLIDGE, ARIZONA  
GA APRON AND TAXIWAY C  
CONTRACT IDENTIFICATION AND NUMBER:  
PROJECT NO. AIP-01-2024**

**1.01** The undersigned Bidder proposes and agrees, if this Bid is accepted, to enter into an Agreement with OWNER in the form included in the Bidding Documents to perform all work as specified or indicated in the Contract Documents for the prices and within the times indicated in this Bid and in accordance with the other terms and conditions of the Bidding Documents.

**2.01** Bidder accepts all of the terms and conditions of the Advertisement or Invitation to Bid and Instructions to Bidders, including without limitation those dealing with the disposition of Bid security. The Bid will remain subject to acceptance for 90 days after the Bid opening, or for such longer period of time that Bidder may agree to in writing upon request of OWNER.

**3.01** In submitting this Bid, Bidder represents, as set forth in the Agreement, that:

A. Bidder has examined and carefully studied the Bidding Documents, the other related data identified in the Bidding Documents, and the following Addenda, receipt of all, which is hereby acknowledged.

<u>Addendum No.</u>	<u>Addendum Date</u>
_____	_____
_____	_____
_____	_____
_____	_____

B. Bidder has visited the Site and become familiar with and is satisfied as to the general, local and Site conditions that may affect cost, progress, and performance of the work.

C. Bidder is familiar with and is satisfied as to all federal, state and local Laws and Regulations that may affect cost, progress and performance of the work.

D. Bidder has carefully studied all: (1) reports of explorations and tests of subsurface conditions at or contiguous to the Site and all drawings of physical conditions in or relating to existing surface or subsurface structures at or contiguous to the Site (except Underground Facilities) which have been identified in the Special Provisions, and (2) reports and drawings of a Hazardous Environmental Condition, if any, which has been identified in the Special Provisions.

E. Bidder has obtained and carefully studied (or assumes responsibility for having done so) all additional or supplementary examinations, investigations, explorations, tests, studies and data concerning conditions (surface, subsurface and Underground Facilities) at or contiguous to the Site which may affect cost, progress, or performance of the Work or which relate to any aspect of the means, methods, techniques, sequences, and procedures of construction to be employed by Bidder, including applying the specific means, methods, techniques, sequences, and procedures of construction expressly required by the Contract Documents to be employed by Bidder, and safety precautions and programs incident thereto.



- F. Bidder does not consider that any further examinations, investigations, explorations, tests, studies, or data are necessary for the determination of this Bid for performance of the Work at the price(s) bid and within the times and in accordance with the other terms and conditions of the Bidding Documents.
- G. Bidder is aware of the general nature of work to be performed by OWNER and others at the Site that relates to the Work as indicated in the Bidding Documents.
- H. Bidder has correlated the information known to Bidder, information and observations obtained from visits to the Site, reports and drawings identified in the Bidding Documents, and all additional examinations, investigations, explorations, tests, studies, and data with the Bidding Documents.
- I. Bidder has given ENGINEER written notice of all conflicts, errors, ambiguities, or discrepancies that Bidder has discovered in the Bidding Documents, and the written resolution thereof by ENGINEER is acceptable to Bidder.
- J. The Bidding Documents are generally sufficient to indicate and convey understanding of all terms and conditions for the performance of the Work for which this Bid is submitted.

**4.01** Bidder further represents that this Bid is genuine and not made in the interest of or on behalf of any undisclosed individual or entity and is not submitted in conformity with any agreement or rules of any group, association, organization or corporation; Bidder has not directly or indirectly induced or solicited any other Bidder to submit a false or sham Bid; Bidder has not solicited or induced any individual or entity to refrain from bidding; and Bidder has not sought by collusion to obtain for itself any advantage over any other Bidder or over OWNER.

**5.01** Bidder will complete the work in accordance with the Contract Documents for the following price(s):



**CONTRACTOR NAME:** \_\_\_\_\_

**BID SCHEDULE**

Base Bid					Unit	
Item No.	Specification Number	Bid Item Description	Quantity	Unit	Price	Amount
1	C-100	Contractor Quality Control	1	LS		
2	C-102-5.1	Storm Water Pollution Prevention	1	LS		
3	C-105	Mobilization/Demobilization (Maximum of 5%)	1	LS		
4	SP-60.1	Airside Safety and Security / Traffic Control	1	LS		
5	P-101-5.1	Bituminous Pavement Removal (4 In. Depth)	130	SY		
6	P-151-4.1	Clearing and Grubbing	5	AC		
7	P-152-4.1	Unclassified Excavation	990	CY		
8	P-152-4.2	Fill with Select Material	5,670	CY		
9	P-152-5.1	Compacted Subgrade (10 In. Depth)	14,690	SY		
10	P-208.1	Aggregate Base Course (Stabilized Shoulder)	300	CY		
11	P-209.1	Crushed Aggregate Base Course (6")	2,450	CY		
12	P-401-8.1	Asphalt Mix Pavement	2,550	TONS		
13	P-610	Aircraft Tie-Down	36	EA		
14	P-620.1	Reflective Yellow Taxiway Pavement Markings, Waterborne	880	SF		
15	P-620.2	Reflective White Runway Pavement Markings, Waterborne	140	SF		
16	P-620.3	Black Taxiway Pavement Markings, Waterborne	450	SF		
17	D-701-5.1	18-inch Concrete Pipe RGRCP, Class V	130	LF		
18	D-751-5.2	Storm Drain Inlet	2	EA		
19	D-751-5.3	Storm Drain Apron	2	EA		
20	MAG 220.5	UngROUTED Rip Rap. D50=9", with Geosynthetic Fabric	150	SY		
21	L-108-5.1	No. 8 AWG, 5 kV, L-824, Type C Cable, Installed in Trench, Duct Bank, or Conduit	2,310	LF		
22	L-108-5.2	No. 6 AWG, Bare CU Counterpoise Installed in Trench	1,670	LF		
23	L-110-5.1	Non-Encased Conduit, 1-Way 2"	1,190	LF		



**CONTRACTOR NAME:** \_\_\_\_\_

**BID SCHEDULE – BASE BID (CONTINUED)**

Base Bid (Continued)					Unit	
Item No.	Specification Number	Bid Item Description	Quantity	Unit	Price	Amount
24	L-110-5.2	Concrete Encased Conduit, 1-Way 2"	170	LF		
25	L-110-5.3	Concrete Encased Conduit, 2-Way 2"	90	LF		
26	L-110-5.4	Directional Bored Duct Bank, 4-Way 2"	220	LF		
27	L-115-5.1	Concrete H-20 Load Rated Electrical Handhole	3	EA		
28	L-125-5.1	NEW L-861T Elevated Taxiway Edge Light Installed on L-867 Base Can including Isolation Transformer, stem, plate and splice kit	30	EA		
29	L-125-5.2	NEW L-861T Elevated Taxiway Edge Light Installed on L-868(B) Base Can including Isolation Transformer, stem, plate and splice kit	1	EA		
30	L-125-5.3	New 2 Module Size 1 LED Airfield Sign with Concrete Base	2	EA		
31	L-125-5.4	New 3 Module Size 1 LED Airfield Sign with Concrete Base	2	EA		
32	L-125-5.5	New 4 Module Size 1 LED Airfield Sign with Concrete Base	1	EA		

**TOTAL BASE BID AMOUNT  
(IN NUMBERS)**

\$ \_\_\_\_\_

**TOTAL BASE BID AMOUNT  
(IN WORDS):**



**CONTRACTOR NAME:** \_\_\_\_\_

**BID SCHEDULE**

Alternative 1 Bid					Unit	
Item No.	Specification Number	Bid Item Description	Quantity	Unit	Price	Amount
1	C-100	Contractor Quality Control	1	LS		
2	C-102-5.1	Storm Water Pollution Prevention	1	LS		
3	C-105	Mobilization/Demobilization (Maximum of 5%)	1	LS		
4	SP-60.1	Airside Safety and Security / Traffic Control	1	LS		
5	P-101-5.1	Bituminous Pavement Removal (4 In. Depth)	140	SY		
5	P-151-4.1	Clearing and Grubbing	1	AC		
6	P-152-4.1	Unclassified Excavation	290	CY		
7	P-152-4.2	Fill with Select Material	270	CY		
8	P-152-5.1	Compacted Subgrade (10 In. Depth)	1,790	SY		
9	P-208.1	Aggregate Base Course (Stabilized Shoulder)	140	CY		
10	P-209.1	Crushed Aggregate Base Course (6")	300	CY		
11	P-401-8.1	Asphalt Mix Pavement	400	TONS		
12	P-620.1	Reflective Yellow Taxiway Pavement Markings, Waterborne	450	SF		
13	P-620.2	Reflective White Runway Pavement Markings, Waterborne	180	SF		
14	P-620.3	Black Taxiway Pavement Markings, Waterborne	410	SF		
15	D-701-5.1	24-inch Concrete Pipe RGRCP, Class V	110	LF		
16	D-751-5.2	Storm Drain Inlet	2	EA		
17	D-751-5.3	Storm Drain Apron	2	EA		
18	MAG 220.5	UngROUTED Rip Rap. D50=9", with Geosynthetic Fabric	50	SY		
19	L-108-5.1	No. 8 AWG, 5 kV, L-824, Type C Cable, Installed in Trench, Duct Bank, or Conduit	1,275	LF		
20	L-108-5.2	No. 6 AWG, Bare CU Counterpoise Installed in Trench	855	LF		
21	L-110-5.1	Non-Encased Conduit, 1-Way 2"	750	LF		
22	L-110-5.2	Concrete Encased Conduit, 1-Way 2"	105	LF		



**CONTRACTOR NAME:** \_\_\_\_\_

**BID SCHEDULE – ALTERNATIVE 1 (CONTINUED)**

Alternative 1 Bid					Unit	
Item No.	Specification Number	Bid Item Description	Quantity	Unit	Price	Amount
23	L-115-5.1	Concrete H-20 Load Rated Electrical Handhole	1	EA		
24	L-125-5.1	NEW L-861T Elevated Taxiway Edge Light Installed on L-867 Base Can including Isolation Transformer, stem, plate and splice kit	18	EA		
25	L-125-5.3	New 2 Module Size 1 LED Airfield Sign with Concrete Base	1	EA		
26	L-125-5.5	New 4 Module Size 1 LED Airfield Sign with Concrete Base	1	EA		

**TOTAL ALTERNATIVE 1 BID AMOUNT  
(IN NUMBERS)**

\$ \_\_\_\_\_

**TOTAL ALTERNATIVE 1 BID AMOUNT  
(IN WORDS):**



**CONTRACTOR NAME:** \_\_\_\_\_

**BID SCHEDULE**

Alternative 2 Bid					Unit	
Item No.	Specification Number	Bid Item Description	Quantity	Unit	Price	Amount
1	C-100	Contractor Quality Control	1	LS		
2	C-102-5.1	Storm Water Pollution Prevention	1	LS		
3	C-105	Mobilization/Demobilization (Maximum of 5%)	1	LS		
4	SP-60.1	Airside Safety and Security / Traffic Control	1	LS		
5	P-152-4.1	Unclassified Excavation	470	CY		
6	P-152-4.2	Fill with Select Material	40	CY		
7	P-152-5.1	Compacted Subgrade (10 In. Depth)	3,060	SY		
8	MAG 702-4.1	Crushed Aggregate Base Course (6") (Not FAA AIP Eligible)	510	CY		
9	MAG 710-5.1	Asphalt Concrete (4") (Not FAA AIP Eligible)	510	TONS		
10	P-620.1	Reflective Yellow Taxiway Pavement Markings, Waterborne	1,740	SF		
11	1-125-5.6	NEW L-861T Elevated Taxiway Edge Light Installed on L-867 Base Can with Base Can Extension including Isolation Transformer, stem, plate and splice kit (price differential from Taxiway Edge Lights specified per Base Bid and Alternate 1)	48	EA		

**TOTAL ALTERNATIVE 2 BID AMOUNT  
(IN NUMBERS)**

\$ \_\_\_\_\_

**TOTAL ALTERNATIVE 2 BID AMOUNT  
(IN WORDS):**



The lower responsible bidder will be as determined from Article 14 – BASIS OF BID; EVALUATION OF BIDS. The award of the contract will be according to Article 19, AWARD OF CONTRACT of the *Instructions to Bidders*.

Unit Prices have been computed in accordance with Article 14 of the *Instructions to Bidders*.

Bidder acknowledges that estimated quantities are not guaranteed and are solely for the purpose of comparison of Bids, and final payment for all Unit Price Bid items will be based on actual quantities constructed and accepted by the Engineer, determined as provided in the Contract Documents.

Bid Schedule Summary:

Total: \$ \_\_\_\_\_

**6.01** Bidder agrees that the work will be substantially completed and ready for final payment in accordance with Article 3 of the Construction Contract, and within the number of calendar days indicated in the Construction Contract.

**7.01** Bidder accepts the provisions of the Agreement (*Construction Contract*) as to liquidated damages in the event of failure to complete the work within the times specified above, which shall be stated in the Agreement.

**8.01** The following documents are attached to and made a condition of this Bid.

- a. Certified Copy of Resolution of Board of Directors (if Bidder is corporation) or evidence of authority to sign (if Bidder is a partnership).
- b. Statutory Bid Bond (if used in lieu of cashier's check or certified check).
- c. Certificate of Insurability.
- d. Non-Collusive Bidding Certification.
- e. Bidders Qualification Statements with supporting data.
- f. A list of Subcontractors that the Bidder proposes to utilize in the contract.
- g. A list of proposed electrical equipment, including the name of equipment and manufacturer, specification item and/or model number and estimated time to deliver to the project site.
- h. Bidder's Statement on Previous Contracts Subject to EEO Clauses.
- i. Buy American Certificate Steel and Manufactured Products for Construction Contracts.
- j. DBE Identification Form
- k. Tax Delinquency and Felony Conviction Certificate
- l. Attachment A, Base Bid, Subcontracting and Good Faith Efforts Summary.
- m. Attachment B, Base Bid, Letter of Intent to perform as a subcontractor/subconsultant/supplier.
- n. Attachment C, where applicable. Identification Statement for Disadvantaged Business Enterprises. Submit for any proposed DBE sub-contractor that is certified but not certified by ADOT, the city of Phoenix or Tucson.

**9.01** The terms used in this Bid with initial capital letters have the meanings indicated in the Instructions to Bidders and the *Special Provisions*.

SUBMITTED on \_\_\_\_\_, 202\_\_.

State Contractor License No. \_\_\_\_\_.



If Bidder is:

An Individual

Name (typed or printed): \_\_\_\_\_

By: \_\_\_\_\_ (SEAL)  
*(Individual's signature)*

Doing business as: \_\_\_\_\_

Business address: \_\_\_\_\_

Phone No.: \_\_\_\_\_ Facsimile No.: \_\_\_\_\_

A Partnership

Partnership Name: \_\_\_\_\_ (SEAL)

By: \_\_\_\_\_  
*(Signature of general partner -- attach evidence of authority to sign)*

Name (typed or printed): \_\_\_\_\_

Business address: \_\_\_\_\_

Phone No.: \_\_\_\_\_ Facsimile No.: \_\_\_\_\_

A Corporation

Corporation Name: \_\_\_\_\_ (SEAL)

State of Incorporation: \_\_\_\_\_

Type (General Business, Professional, Service, Limited Liability): \_\_\_\_\_

By: \_\_\_\_\_  
*(Signature -- attach evidence of authority to sign)*

Name (typed or printed): \_\_\_\_\_

Title: \_\_\_\_\_

Attest \_\_\_\_\_  
*(Signature of Corporate Secretary)*

Business address: \_\_\_\_\_

Phone No.: \_\_\_\_\_ Facsimile No.: \_\_\_\_\_

Date of Qualification to do business is \_\_\_\_\_.



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**A Joint Venture**

Joint Venture Name: \_\_\_\_\_ (SEAL)

By: \_\_\_\_\_  
*(Signature of joint venture partner -- attach evidence of authority to sign)*

Name (typed or printed): \_\_\_\_\_

Title: \_\_\_\_\_

Business address: \_\_\_\_\_

Phone No.: \_\_\_\_\_ Facsimile No.: \_\_\_\_\_

Joint Venture Name: \_\_\_\_\_ (SEAL)

By: \_\_\_\_\_  
*(Signature -- attach evidence of authority to sign)*

Name (typed or printed): \_\_\_\_\_

Title: \_\_\_\_\_

Business address: \_\_\_\_\_

Phone No.: \_\_\_\_\_ Facsimile No.: \_\_\_\_\_

Phone and facsimile number, and address for receipt of official communications:  
\_\_\_\_\_  
\_\_\_\_\_  
\_\_\_\_\_

(Each joint venture must sign. The manner of signing for each individual, partnership, and corporation that is a party to the joint venture should be in the manner indicated above.)



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**CERTIFIED COPY OF RESOLUTION OF  
BOARD OF DIRECTORS OF**

**(Name of Corporation)**

RESOLVED that \_\_\_\_\_,  
(Person Authorized to Sign) (Title)

of \_\_\_\_\_ be authorized to sign and submit the Bid

of \_\_\_\_\_  
(Name of Corporation)

proposal of this corporation for the following projects:

- GA Apron and Taxiway C**
- Project No. AIP-01-2024**
- ADOT**
- FAA AIP 3-04-0011-018-2025 (Design)**
- FAA IIJA 3-04-0011-019-2025**

The foregoing is a true and correct copy of the resolution adopted by \_\_\_\_\_  
\_\_\_\_\_ at the meeting of its City Council held on the \_\_\_\_\_ day of \_\_\_\_\_, 202\_\_.

By: \_\_\_\_\_

Title: \_\_\_\_\_

(SEAL)



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**STATUTORY BID BOND**

**KNOW ALL MEN BY THESE PRESENTS:**

That, \_\_\_\_\_, (hereinafter called the Principal), as Principal, and \_\_\_\_\_, a corporation duly organized under the laws of the State of \_\_\_\_\_, with its principal office in the City of \_\_\_\_\_, (hereinafter called the Surety), as Surety, are held and firmly bound unto City of Coolidge, Pinal County, Arizona (hereinafter called the Obligee), in the penal sum of **Ten Percent (10%)** of the total amount of Principal’s bid in lawful money of the United States of America, for the payment of which the Principal and Surety bind themselves and their heirs, administrators, executors, successors, and assigns, jointly and severally, firmly by these presents.

WHEREAS, the Obligee advertised for bids to construct GA Apron and Taxiway C, FAA AIP 3-04-0011-018-2025, ADOT No., Coolidge Project No. AIP-01-2024, and the Principal submitted a bid/proposal to construct the improvements,

NOW, THEREFORE, THE CONDITION OF THIS OBLIGATION IS SUCH, that if the Obligee accepts the proposal of the Principal and the Principal enters into a contract with the Obligee in accordance with the terms of the proposal and gives the bonds and certificates of insurance as specified in the standard specifications with good and sufficient surety for the faithful performance of the contract and for the prompt payment of labor and materials furnished in the prosecution of the contract, or in the event of the failure of the Principal to enter into the contract and give the bonds and certificates of insurance, if the Principal pays to the Obligee the difference not to exceed the penalty of the bond between the amount specified in the proposal and such larger amount for which the Obligee may in good faith contract with another party to perform the work covered by the proposal then this obligation is void. Otherwise it remains in full force and effect provided, however, that this bond is executed pursuant to the provisions of § 34-201, Arizona Revised Statutes, and all liabilities on this bond shall be determined in accordance with the provisions of the section to the extent as if it were copied at length herein.

Witness our hands this \_\_\_\_\_ day of \_\_\_\_\_, 202\_\_.

\_\_\_\_\_  
**AGENCY OF RECORD, STATE OF ARIZONA**

\_\_\_\_\_  
**PRINCIPAL**

BY: \_\_\_\_\_

\_\_\_\_\_  
**AGENCY ADDRESS**

\_\_\_\_\_  
**TITLE:**

\_\_\_\_\_  
**SURETY**

BY: \_\_\_\_\_

TITLE: \_\_\_\_\_

**BOND NUMBER:** \_\_\_\_\_

**ATTACH SURETY POWER OF ATTORNEY**



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### CERTIFICATE OF INSURABILITY

I hereby certify that as a Bidder for this project, I am fully aware of the insurance requirements for the Contractor and that by submitting this bid proposal, assure the Owner that I am able to produce the required minimum insurance coverage should I be selected to be the successful bidder.

Should I be selected to be the successful bidder and then become unable to produce the insurance coverage prior to the award of the project, I understand that my bid will be rejected and that I will forfeit by bid bond.

\_\_\_\_\_ Date: \_\_\_\_\_  
COUNTERSIGNED BY (Insurance Representative)

\_\_\_\_\_ Date: \_\_\_\_\_  
AUTHORIZED CONTRACTOR'S REPRESENTATIVE (Signature)

\_\_\_\_\_  
(Firm's Name)

\_\_\_\_\_  
(Title)



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**NON-COLLUSIVE BIDDING CERTIFICATION**

STATE OF )  
 ) ss  
COUNTY OF )

I, \_\_\_\_\_ of the City of \_\_\_\_\_, in the County of \_\_\_\_\_ and the State of \_\_\_\_\_, of full age, being duly sworn according to the law on my oath depose and say that:

I am \_\_\_\_\_, (name) a \_\_\_\_\_

(Title, Position, Etc.) of the firm of \_\_\_\_\_, the Bidder making the bid for the proposed project as described in this set of contract documents, and that I executed the said Bid with full authority so to do; that said Bidder as not, directly or indirectly, entered into any agreement, participated in any collusion, or otherwise taken any action in restraint of free, competitive bidding in connection with the above name Project; and that all statements contained in said Bid and in this affidavit are true and correct, and made with full knowledge that the Owner relies upon the truth of the statements contained in said Bid and in the statements contained in this affidavit is awarding the Contract for the said Project.

I further warrant that no person or selling agency has been employed or retained to solicit or secure such Contract upon an agreement of understanding, for commission, percentage, brokerage or contingent fee, except bona fide employees or bona fide established commercial or selling agencies maintained by:

\_\_\_\_\_  
Signature of Bidder

\_\_\_\_\_  
Printed or Typed Name of Bidder

\_\_\_\_\_  
Seal if Corporation

Sworn to before me this \_\_\_\_\_ day of \_\_\_\_\_, 2014, in the County of \_\_\_\_\_, State of \_\_\_\_\_.

\_\_\_\_\_  
Notary Public



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### BIDDERS QUALIFICATION STATEMENTS

(Completion of this Statement is Required for Advance of Consideration for Award of Contract)

SUBMITTED BY:

Name: \_\_\_\_\_  
(Print or Type Name of Bidder)

(A Corporation/A Partnership/An Individual/A Joint Venture)  
([Bidder to strike out non-applicable terms])

Address: \_\_\_\_\_  
\_\_\_\_\_  
\_\_\_\_\_

Gentlemen:

The undersigned certifies under oath the truth and correctness of all statements and of all answers to questions made hereinafter.

(Note: Attach Separate Sheets as Required)

- 1.0 How many years has your organization been in business as a Contractor? \_\_\_\_\_
- 2.0 How many years has your organization been in business under its present name? \_\_\_\_\_
- 3.0 If a corporation, answer the following:
  - 3.1 Date of incorporation \_\_\_\_\_
  - 3.2 State of incorporation: \_\_\_\_\_
  - 3.3 President's name: \_\_\_\_\_
  - 3.4 Vice president's name(s): \_\_\_\_\_
  - 3.5 Secretary or Clerk's name: \_\_\_\_\_
  - 3.6 Treasurer's name: \_\_\_\_\_

- 4.0 If individual or partnership, answer the following:
  - 4.1 Date of organization: \_\_\_\_\_
  - 4.2 Name and address of all partners. (State whether general or limited partnership):  
\_\_\_\_\_  
\_\_\_\_\_  
\_\_\_\_\_

5.0 If other than corporation or partnership, describe organization and name principals: \_\_\_\_\_  
\_\_\_\_\_

6.0 Do you plan to subcontract any part of this project? \_\_\_\_\_.  
If yes, Bidder shall complete the List of Subcontractors Form provided herein along with any required applicable DBE subcontract forms provided herein.

7.0 Has any construction contract to which you have been a party been terminated by the owner; have you ever terminated a project prior to its completion for any reason; has any surety which issued a performance bond on your



behalf ever completed the work in its own name or financed such completion on your behalf; has any surety expended any monies in connection with a contract for which they furnished a bond on your behalf? If the answer to any portion of this question is "yes", please furnish details of all such occurrences including name of owner, architect or engineer, and surety, and name and date of project.

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8.0 Has any officer or partner of your organization ever been an officer or partner of another organization that had any construction contract terminated by the owner; terminated work on a project prior to its completion for any reason; had any surety which issued a performance bond complete the work in its own name or financed such completion; or had any surety expend any monies in connection with a contract for which they furnished a bond? If the answer to any portion of this question is "yes", please furnish details of all such occurrences including name of owner, architect or engineer, and surety, and name and date of project.

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9.0 List name of project, owner, architect or engineer, contract amount, percent complete and schedule completion of the major construction projects your organization has in process on this date.

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10.0 List name of project, owner, architect or engineer, contract amount, date of completion and percent of work completed with your own forces for project of the same general nature as this project that your organization has completed in the past seven years, (reference Article 3.2 on page I-2 for project qualification requirements).

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11.0 List name, address and telephone number of a reference for each project listed under Items 9.0 and 10.0, above.

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12.0 List name and construction experience of the principal individuals of your organization.

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13.0 List the states and categories of construction in which your organization is legally qualified to do business.

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14.0 List name, address and telephone number of an individual who represents each of the following and whom OWNER may contact for a financial reference:

- 14.1 A Surety \_\_\_\_\_
- 14.2 A bank: \_\_\_\_\_
- 14.3 A major potential supplier: \_\_\_\_\_



15.0 Attach a financial statement, prepared on an accrual basis, in a form that clearly indicates Bidder's assets, liabilities and net worth. Evidence that Bidder has been prequalified with the Arizona State Highway Division and is on their current "Bidder's List" may not be submitted in lieu of the certified financial statement.

- 15.1 Date of financial statement \_\_\_\_\_
- 15.2 Name of firm preparing statement: \_\_\_\_\_

16.0 The Contractor shall provide to the Engineer, a list of all proposed electrical equipment, including the name of equipment and manufacture, specification item and/or model number and estimated time for delivery to the project site within (7) calendar days of the Notice of Intent to Award, see Article 12.6 for further information (page I-6).

17.0 Dated at \_\_\_\_\_ this \_\_\_\_\_ day of \_\_\_\_\_, 202\_\_.

(Print or Type Name of Bidder)

(Seal, if corporation) By \_\_\_\_\_ Title



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**BIDDER'S STATEMENT ON PREVIOUS CONTRACTS  
SUBJECT TO EEO CLAUSE**

The Bidder shall complete the following statement by checking the appropriate boxes.

The Bidder has \_\_\_ has not \_\_\_ participated in a previous contract subject to the equal opportunity clause prescribed by Executive Order 11246, as amended, of September 24, 1965.

The Bidder has \_\_\_ has not \_\_\_ submitted all compliance reports in connection with any such contract due under the applicable filing requirements; and that presentations indicating submission of required compliance reports signed by proposed subcontractors will be obtained prior to award of subcontracts.

If the Bidder has participated in a previous contract subject to the equal opportunity clause and has not submitted compliance reports due under application filing requirements, the Bidder shall submit a compliance report on Standard Form 100, "Employee Information Report EEO-I" prior to award of contract.

\_\_\_\_\_  
Signature

\_\_\_\_\_  
Name of Bidder

\_\_\_\_\_  
Date

\_\_\_\_\_  
Name and Title of Signing Officer

Business Address: \_\_\_\_\_

\_\_\_\_\_

\_\_\_\_\_

NOTE: Failure to complete the blanks may be grounds for rejection of the bid.



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**BIDDER'S STATEMENT ON PREVIOUS CONTRACTS  
SUBJECT TO EEO CLAUSE**

The Bidder shall complete the following statement by checking the appropriate boxes.

The Bidder has \_\_\_ has not \_\_\_ participated in a previous contract subject to the equal opportunity clause prescribed by Executive Order 11246, as amended, of September 24, 1965.

The Bidder has \_\_\_ has not \_\_\_ submitted all compliance reports in connection with any such contract due under the applicable filing requirements; and that presentations indicating submission of required compliance reports signed by proposed subcontractors will be obtained prior to award of subcontracts.

If the Bidder has participated in a previous contract subject to the equal opportunity clause and has not submitted compliance reports due under application filing requirements, the Bidder shall submit a compliance report on Standard Form 100, "Employee Information Report EEO-I" prior to award of contract.

See also Division III – *Federal Assurance Requirements*.

\_\_\_\_\_  
Signature

\_\_\_\_\_  
Name of Bidder

\_\_\_\_\_  
Date

\_\_\_\_\_  
Name and Title of Signing Officer

Business Address: \_\_\_\_\_  
\_\_\_\_\_  
\_\_\_\_\_

NOTE: Failure to complete the blanks may be grounds for rejection of the bid.



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## BUY AMERICAN CERTIFICATION

As a matter of bid responsiveness, the bidder or offeror must complete, sign, date, and submit this certification statement with their proposal. The bidder or offeror must indicate how they intend to comply with 49 USC § 50101 by selecting one of the following certification statements. These statements are mutually exclusive. Bidder must select one or the other (not both) by inserting a checkmark (✓) or the letter “X”. See Division III, Federal Assurance Requirements for further information.

### Certificate of Buy American Compliance for Manufactured Products

As a matter of bid responsiveness, the bidder or offeror must complete, sign, date, and submit this certification statement with their proposal. The bidder or offeror must indicate how they intend to comply with 49 USC § 50101 by selecting one on the following certification statements. These statements are mutually exclusive. Bidder must select one or the other (not both) by inserting a checkmark (✓) or the letter “X”.

- Bidder or offeror hereby certifies that it will comply with 49 USC § 50101 by:
- Only installing steel and manufactured products produced in the United States, or;
  - Installing manufactured products for which the FAA has issued a waiver as indicated by inclusion on the current FAA Nationwide Buy American Waivers Issued listing, or;
  - Installing products listed as an Excepted Article, Material or Supply in Federal Acquisition Regulation Subpart 25.108.

By selecting this certification statement, the bidder or offeror agrees:

- To provide to the Owner evidence that documents the source and origin of the steel and manufactured product.
- To faithfully comply with providing US domestic product
- To furnish US domestic product for any waiver request that the FAA rejects
- To refrain from seeking a waiver request after establishment of the contract, unless extenuating circumstances emerge that the FAA determines justified.

- The bidder or offeror hereby certifies it cannot comply with the 100% Buy American Preferences of 49 USC § 50101(a) but may qualify for either a Type 3 or Type 4 waiver under 49 USC § 50101(b). By selecting this certification statement, the apparent bidder or offeror with the apparent low bid agrees:
- To the submit to the Owner within 15 calendar days of the bid opening, a formal waiver request and required documentation that support the type of waiver being requested.
  - That failure to submit the required documentation within the specified timeframe is cause for a non-responsive determination may result in rejection of the proposal.
  - To faithfully comply with providing US domestic products at or above the approved US domestic content percentage as approved by the FAA.
  - To refrain from seeking a waiver request after establishment of the contract, unless extenuating circumstances emerge that the FAA determines justified.

### Required Documentation

**Type 3 Waiver** - The cost of the item components and subcomponents produced in the United States is more that 60% of the cost of all components and subcomponents of the “item”. The required documentation for a type 3 waiver is:

- Listing of all product components and subcomponents that are not comprised of 100% US domestic content (Excludes products listed on the FAA Nationwide Buy American Waivers



Issued listing and products excluded by Federal Acquisition Regulation Subpart 25.108; products of unknown origin must be considered as non-domestic products in their entirety).

- b) Cost of non-domestic components and subcomponents, excluding labor costs associated with final assembly at place of manufacture.
- c) Percentage of non-domestic component and subcomponent cost as compared to total “item” component and subcomponent costs, excluding labor costs associated with final assembly at place of manufacture.

**Type 4 Waiver** – Total cost of project using US domestic source product exceeds the total project cost using non-domestic product by 25%. The required documentation for a type 4 of waiver is:

- a) Detailed cost information for total project using US domestic product
- b) Detailed cost information for total project using non-domestic product

**False Statements:** Per 49 USC § 47126, this certification concerns a matter within the jurisdiction of the Federal Aviation Administration and the making of a false, fictitious or fraudulent certification may render the maker subject to prosecution under Title 18, United States Code.

\_\_\_\_\_ Date

\_\_\_\_\_ Signature

\_\_\_\_\_ Company Name

\_\_\_\_\_ Title



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### BIDDER'S DBE IDENTIFICATION FORM

Owner is required to obtain the following information from each Bidder at time of bid submittal, creating an obligation for the Bidder to provide such information.

**TO BE COMPLETED BY AND FOR BIDDER ONLY (NOT SUBCONTRACTORS)**

Name of Bidder's Firm: \_\_\_\_\_

Bidder Address: \_\_\_\_\_  
\_\_\_\_\_  
\_\_\_\_\_

**At time of IFB submittal, Bidder  is OR  is not a certified DBE**

*DBE is defined as a small business concern that has successfully completed a DBE certification process and been granted DBE status by the Arizona Unified Certification Program or by a U.S. Department of Transportation (USDOT) recognized agency who certifies DBE applicants pursuant to the criteria contained in 49 CFR Part 26.*

Age of Bidder's Firm:  Less than 1 year  
 1 – 3 years  
 4 – 7 years  
 8 – 10 years  
 More than 10 years

Annual Gross Receipts of Bidder's Firm:  Less than \$500,000.00  
 \$500,000.00 - \$1,000,000.00  
 \$1,000,001.00 - \$2,000,000.00  
 \$2,000,001.00 - \$5,000,000.00  
 Greater than \$5,000,000.00

**I DECLARE UNDER PENALTY OF PERJURY IN THE SECOND DEGREE, AND ANY OTHER APPLICABLE STATE OF FEDERAL LAWS, THE STATEMENTS MADE ON THIS DOCUMENT ARE TRUE AND COMPLETE TO THE BEST OF MY KNOWLEDGE.**

SIGNED AND DATED this \_\_\_\_\_ day of \_\_\_\_\_, 202\_\_

\_\_\_\_\_  
Authorized Signature

\_\_\_\_\_  
Print Name and Title



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### TAX DELINQUENCY AND FELONY CONVICTION CERTIFICATION

As a matter of bid responsiveness, the bidder or offeror must complete, sign, date, and submit this certification statement with their proposal. The bidder or offeror must indicate its current status as it relates to tax delinquency and felony conviction by inserting a checkmark (✓) in the space following the applicable response.

The bidder agrees that, if awarded a contract resulting from this solicitation, it will incorporate this required provision for certification in all lower tier subcontracts

See Division III, Federal Assurance Requirements for Felony Conviction and Tax Delinquency definitions and further information.

- 1) The applicant represents that it is  is not  a corporation that has any unpaid Federal tax liability that has been assessed, for which all judicial and administrative remedies have been exhausted or have lapsed, and that is not being paid in a timely manner pursuant to an agreement with the authority responsible for collecting the tax liability.
  
- 2) The applicant represents that it is  is not  a corporation that was convicted of a criminal violation under any Federal law within the preceding 24 months.

SIGNED AND DATED this \_\_\_\_\_ day of \_\_\_\_\_, 202\_\_

\_\_\_\_\_  
Authorized Signature

\_\_\_\_\_  
Print Name and Title



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**ATTACHMENT II  
SPECS DIVISION V - TECHNICAL  
SPECIFICATIONS**

**P-209 CRUSHED AGGREGATE  
BASE COURSE**



## Item P-209 Crushed Aggregate Base Course

### DESCRIPTION

**209-1.1** This item consists of a base course composed of crushed aggregate base constructed on a prepared course in accordance with these specifications and in conformity to the dimensions and typical cross-sections shown on the plans.

### MATERIALS

**209-2.1 Crushed aggregate base.** Crushed aggregate shall consist of clean, sound, durable particles of crushed stone, crushed gravel, and shall be free from coatings of clay, silt, organic material, clay lumps or balls or other deleterious materials or coatings. The method used to produce the crushed gravel shall result in the fractured particles in the finished product as consistent and uniform as practicable. Fine aggregate portion, defined as the portion passing the No. 4 (4.75 mm) sieve shall consist of fines from the coarse aggregate crushing operation. The fine aggregate shall be produced by crushing stone, gravel, that meet the coarse aggregate requirements for wear and soundness. Aggregate base material requirements are listed in the following table.

**Crushed Aggregate Base Material Requirements**

Material Test	Requirement	Standard
<b>Coarse Aggregate</b>		
Resistance to Degradation	Loss: 45% maximum	ASTM C131
Soundness of Aggregates by Use of Sodium Sulfate or Magnesium Sulfate	Loss after 5 cycles: 12% maximum using Sodium sulfate - or - 18% maximum using magnesium sulfate	ASTM C88
Percentage of Fractured Particles	Minimum 90% by weight of particles with at least two fractured faces and 98% with at least one fractured face <sup>1</sup>	ASTM D5821
Flat Particles, Elongated Particles, or Flat and Elongated Particles	10% maximum, by weight, of flat, elongated, or flat and elongated particles <sup>2</sup>	ASTM D4791
<b>Fine Aggregate</b>		
Liquid limit	Less than or equal to 25	ASTM D4318
Plasticity Index	Not more than five (5)	ASTM D4318

<sup>1</sup> The area of each face shall be equal to at least 75% of the smallest mid-sectional area of the piece. When two fractured faces are contiguous, the angle between the planes of fractures shall be at least 30 degrees to count as two fractured faces.

<sup>2</sup> A flat particle is one having a ratio of width to thickness greater than five (5); an elongated particle is one having a ratio of length to width greater than five (5).



**209-2.2 Gradation requirements.** The gradation of the aggregate base material shall meet the requirements of the gradation given in the following table when tested per ASTM C117 and ASTM C136. The gradation shall be well graded from coarse to fine and shall not vary from the lower limit on one sieve to the high limit on an adjacent sieve or vice versa.

**Gradation of Aggregate Base**

Sieve Size	Design Range Percentage by Weight passing	Contractor's Final Gradation	Job Control Grading Band Tolerances <sup>1</sup> (Percent)
2 inch (50 mm)	100		0
1-1/2 inch (37.5 mm)	95-100		±5
1 inch (25.0 mm)	70-95		±8
3/4 inch (19.0 mm)	55-85		±8
No. 4 (4.75 mm)	30-60		±8
No. 40 <sup>2</sup> (425 µm)	10-30		±5
No. 200 <sup>2</sup> (75 µm)	0-10		±3

<sup>1</sup> The “Job Control Grading Band Tolerances for Contractor’s Final Gradation” in the table shall be applied to “Contractor’s Final Gradation” to establish a job control grading band. The full tolerance still applies if application of the tolerances results in a job control grading band outside the design range.

<sup>2</sup> The fraction of material passing the No 200 (75 µm) sieve shall not exceed two-thirds the fraction passing the No 40 (425 µm) sieve.

**209-2.3 Sampling and Testing.**

**a. Aggregate base materials.** The Contractor shall take samples of the aggregate base in accordance with ASTM D75 to verify initial aggregate base requirements and gradation. Material shall meet the requirements in paragraph 209-2.1. This sampling and testing will be the basis for approval of the aggregate base quality requirements.

**b. Gradation requirements.** The Contractor shall take at least two aggregate base samples per day in the presence of the Resident Project Representative (RPR) to check the final gradation. Sampling shall be per ASTM D75. Material shall meet the requirements in paragraph 209-2.2. The samples shall be taken from the in-place, un-compacted material at sampling points and intervals designated by the RPR.

**209-2.4 Separation Geotextile.** Not used.



## CONSTRUCTION METHODS

**209-3.1 Control strip.** The first half-day of construction shall be considered the control strip. The Contractor shall demonstrate, in the presence of the RPR, that the materials, equipment, and construction processes meet the requirements of the specification. The sequence and manner of rolling necessary to obtain specified density requirements shall be determined. The maximum compacted thickness may be increased to a maximum of 12 inches (300 mm) upon the Contractor's demonstration that approved equipment and operations will uniformly compact the lift to the specified density. The RPR must witness this demonstration and approve the lift thickness prior to full production.

Control strips that do not meet specification requirements shall be reworked, re-compacted or removed and replaced at the Contractor's expense. Full operations shall not continue until the control strip has been accepted by the RPR. The Contractor shall use the same equipment, materials, and construction methods for the remainder of construction, unless adjustments made by the Contractor are approved by the RPR.

**209-3.2 Preparing underlying subgrade and/or subbase.** The underlying subgrade and/or subbase shall be checked and accepted by the RPR before base course placing and spreading operations begin. Re-proof rolling of the subgrade or proof rolling of the subbase in accordance with Item P-152, at the Contractor's expense, may be required by the RPR if the Contractor fails to ensure proper drainage or protect the subgrade and/or subbase. Any ruts or soft, yielding areas due to improper drainage conditions, hauling, or any other cause, shall be corrected before the base course is placed. To ensure proper drainage, the spreading of the base shall begin along the centerline of the pavement on a crowned section or on the high side of the pavement with a one-way slope.

**209-3.3 Production.** The aggregate shall be uniformly blended and, when at a satisfactory moisture content per paragraph 209-3.5, the approved material may be transported directly to the placement.

**209-3.4 Placement.** The aggregate shall be placed and spread on the prepared underlying layer by spreader boxes or other devices as approved by the RPR, to a uniform thickness and width. The equipment shall have positive thickness controls to minimize the need for additional manipulation of the material. Dumping from vehicles that require re-handling shall not be permitted. Hauling over the uncompacted base course shall not be permitted.

The aggregate shall meet gradation and moisture requirements prior to compaction. The base course shall be constructed in lifts as established in the control strip, but not less than 4 inches (100 mm) nor more than 12 inches (300 mm) of compacted thickness.

When more than one lift is required to establish the layer thickness shown on the plans, the construction procedure described here shall apply to each lift. No lift shall be covered by subsequent lifts until tests verify that compaction requirements have been met. The Contractor shall rework, re-compact and retest any material placed which does not meet the specifications at the Contractor's expense.

**209-3.5 Compaction.** Immediately after completion of the spreading operations, compact each layer of the base course, as specified, with approved compaction equipment. The number, type, and weight of rollers shall be sufficient to compact the material to the required density within the same day that the aggregate is placed on the subgrade.

The field density of each compacted lift of material shall be at least 100% of the maximum density of laboratory specimens prepared from samples of the base material delivered to the jobsite. The



laboratory specimens shall be compacted and tested in accordance with ASTM D1557. The moisture content of the material during placing operations shall be within  $\pm 2$  percentage points of the optimum moisture content as determined by ASTM D698. Maximum density refers to maximum dry density at optimum moisture content unless otherwise specified.

**209-3.6 Weather limitations.** Material shall not be placed unless the ambient air temperature is at least 40°F (4°C) and rising. Work on base course shall not be conducted when the subgrade or subbase is wet or frozen or the base material contains frozen material.

**209-3.7 Maintenance.** The base course shall be maintained in a condition that will meet all specification requirements. When material has been exposed to excessive rain, snow, or freeze-thaw conditions, prior to placement of additional material, the Contractor shall verify that materials still meet all specification requirements. Equipment may be routed over completed sections of base course, provided that no damage results and the equipment is routed over the full width of the completed base course. Any damage resulting to the base course from routing equipment over the base course shall be repaired by the Contractor at the Contractor's expense.

**209-3.8 Surface tolerances.** After the course has been compacted, the surface shall be tested for smoothness and accuracy of grade and crown. Any portion lacking the required smoothness or failing in accuracy of grade or crown shall be scarified to a depth of at least 3 inches (75 mm), reshaped and recompact to grade until the required smoothness and accuracy are obtained and approved by the RPR. Any deviation in surface tolerances shall be corrected by the Contractor at the Contractor's expense. The smoothness and accuracy requirements specified here apply only to the top layer when base course is constructed in more than one layer.

**a. Smoothness.** The finished surface shall not vary more than 3/8-inch (9 mm) when tested with a 12-foot (3.7-m) straightedge applied parallel with and at right angles to the centerline. The straightedge shall be moved continuously forward at half the length of the 12-foot (3.7-m) straightedge for the full length of each line on a 50-foot (15-m) grid.

**b. Grade.** The grade and crown shall be measured on a 50-foot (15-m) grid and shall be within +0 and -1/2 inch (12 mm) of the specified grade.

**209-3.9 Acceptance sampling and testing.** Crushed aggregate base course shall be accepted for density and thickness on an area basis. Two tests shall be made for density and thickness for each 1200 square yds (1000 m<sup>2</sup>). Sampling locations will be determined on a random basis per ASTM D3665

**a. Density.** The Contractor's laboratory shall perform all density tests in the RPR's presence and provide the test results upon completion to the RPR for acceptance.

Each area shall be accepted for density when the field density is at least 100% of the maximum density of laboratory specimens compacted and tested per ASTM D698. The in-place field density shall be determined per ASTM D6938 using Procedure A, the direct transmission method, and ASTM D6938 shall be used to determine the moisture content of the material. The machine shall be calibrated in accordance with ASTM D6938. If the specified density is not attained, the area represented by the failed test must be reworked and/or recompact and two additional random tests made. This procedure shall be followed until the specified density is reached. Maximum density refers to maximum dry density at optimum moisture content unless otherwise specified.

**b. Thickness.** Depth tests shall be made by test holes at least 3 inches (75 mm) in diameter that extend through the base. The thickness of the base course shall be within +0 and -1/2 inch (12 mm) of the



specified thickness as determined by depth tests taken by the Contractor in the presence of the RPR for each area. Where the thickness is deficient by more than 1/2-inch (12 mm), the Contractor shall correct such areas at no additional cost by scarifying to a depth of at least 3 inches (75 mm), adding new material of proper gradation, and the material shall be blended and recompact to grade. The Contractor shall replace, at his expense, base material where depth tests have been taken.

### METHOD OF MEASUREMENT

**209-4.1** The quantity of crushed aggregate base course will be determined by measurement of the number of cubic yards (cubic meters) of material actually constructed and accepted by the RPR as complying with the plans and specifications. Base materials shall not be included in any other excavation quantities.

### BASIS OF PAYMENT

**209-5.1** Payment shall be made at the contract unit price per cubic yard (cubic meter) for crushed aggregate base course. This price shall be full compensation for furnishing all materials, for preparing and placing these materials, and for all labor, equipment tools, and incidentals necessary to complete the item.

Payment will be made under:

Item P-209-5.1 Crushed Aggregate Base Course - per cubic yard (cubic meter)

### REFERENCES

The publications listed below form a part of this specification to the extent referenced. The publications are referred to within the text by the basic designation only.

ASTM International (ASTM)

ASTM C29	Standard Test Method for Bulk Density (“Unit Weight”) and Voids in Aggregate
ASTM C88	Standard Test Method for Soundness of Aggregates by Use of Sodium Sulfate or Magnesium Sulfate
ASTM C117	Standard Test Method for Materials Finer than 75- $\mu$ m (No. 200) Sieve in Mineral Aggregates by Washing
ASTM C131	Standard Test Method for Resistance to Degradation of Small-Size Coarse Aggregate by Abrasion and Impact in the Los Angeles Machine
ASTM C136	Standard Test Method for Sieve or Screen Analysis of Fine and Coarse Aggregates
ASTM C142	Standard Test Method for Clay Lumps and Friable Particles in Aggregates
ASTM D75	Standard Practice for Sampling Aggregates



- ASTM D698 Standard Test Methods for Laboratory Compaction Characteristics of Soil Using Standard Effort (12,400 ft-lbf/ft<sup>3</sup> (600 kN-m/m<sup>3</sup>))
- ASTM D1556 Standard Test Method for Density and Unit Weight of Soil in Place by the Sand-Cone Method
- ASTM D1557 Standard Test Methods for Laboratory Compaction Characteristics of Soil Using Modified Effort (56,000 ft-lbf/ft<sup>3</sup> (2700 kN-m/m<sup>3</sup>))
- ASTM D2167 Standard Test Method for Density and Unit Weight of Soil in Place by the Rubber Balloon Method
- ASTM D2419 Standard Test Method for Sand Equivalent Value of Soils and Fine Aggregate
- ASTM D3665 Standard Practice for Random Sampling of Construction Materials
- ASTM D4318 Standard Test Methods for Liquid Limit, Plastic Limit, and Plasticity Index of Soils
- ASTM D4491 Standard Test Methods for Water Permeability of Geotextiles by Permittivity
- ASTM D4643 Standard Test Method for Determination of Water Content of Soil and Rock by Microwave Oven Heating
- ASTM D4751 Standard Test Methods for Determining Apparent Opening Size of a Geotextile
- ASTM D4791 Standard Test Method for Flat Particles, Elongated Particles, or Flat and Elongated Particles in Coarse Aggregate
- ASTM D5821 Standard Test Method for Determining the Percentage of Fractured Particles in Coarse Aggregate
- ASTM D6938 Standard Test Method for In-Place Density and Water Content of Soil and Soil-Aggregate by Nuclear Methods (Shallow Depth)
- ASTM D7928 Standard Test Method for Particle-Size Distribution (Gradation) of Fine-Grained Soils Using the Sedimentation (Hydrometer) Analysis
- American Association of State Highway and Transportation Officials (AASHTO)
- M288 Standard Specification for Geosynthetic Specification for Highway Applications

**END OF ITEM P-209**



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# **ATTACHMENT III**

## **PLAN SHEETS**



**BASE BID QUANTITIES**

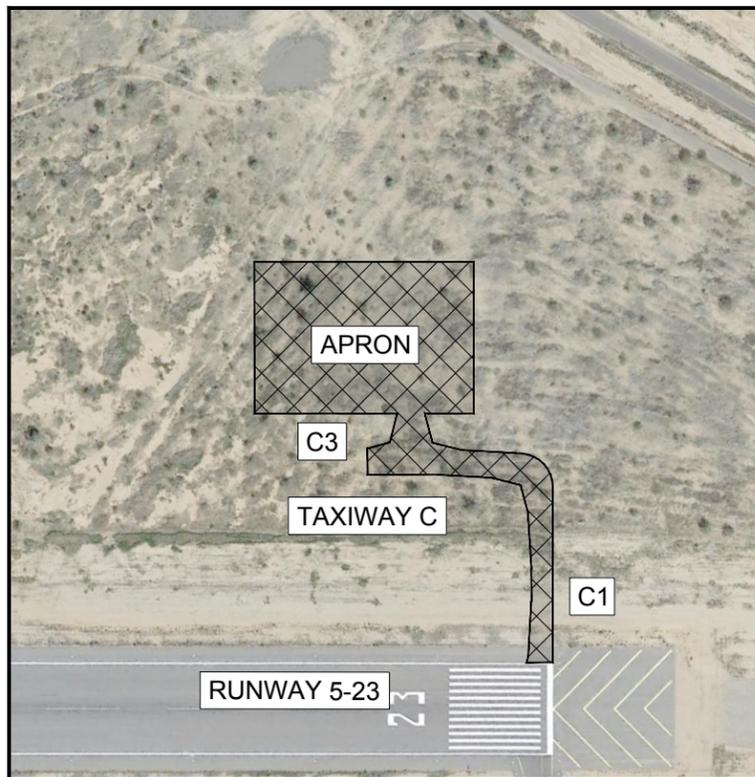
Item No.	Specification Number	Bid Item Description	Quantity	Unit
1	C-100	Contractor Quality Control	1	LS
2	C-102-5.1	Storm Water Pollution Prevention	1	LS
3	C-105	Mobilization/Demobilization (Maximum of 5%)	1	LS
4	SP-60.1	Airside Safety and Security / Traffic Control	1	LS
5	P-101-5.1	Bituminous Pavement Removal (4 In. Depth)	130	SY
6	P-151-4.1	Clearing and Grubbing	5	AC
7	P-152-4.1	Unclassified Excavation	990	CY
8	P-152-4.2	Fill with Select Material	5,670	CY
9	P-152-5.1	Compacted Subgrade (10 In. Depth)	14,690	SY
10	P-208.1	Aggregate Base Course (Stabilized Shoulder) (6")	300	CY
11	P-209.1	Crushed Aggregate Base Course (6")	2,450	CY
13	P-401-8.1	Asphalt Mix Pavement	2,550	TONS
15	P-610	Aircraft Tie-Down	36	EA
16	P-620.1	Reflective Yellow Taxiway Pavement Markings, Waterborne	880	SF
17	P-620.2	Reflective White Runway Pavement Markings, Waterborne	140	SF
18	P-620.3	Black Taxiway Pavement Markings, Waterborne	450	SF
19	D-701-5.1	18-inch Concrete Pipe RGRCP, Class V	130	LF
20	D-751-5.2	Storm Drain Inlet	2	EA
21	D-751-5.3	Storm Drain Apron	2	EA
22	MAG 220.5	Ungrouted Rip Rap, D50=9", with Geosynthetic Fabric No. 8 AWG, 5 kV, L-824, Type C Cable, Installed in Trench, Duct Bank, or Conduit	150	SY
23	L-108-5.1	No. 8 AWG, 5 kV, L-824, Type C Cable, Installed in Trench, Duct Bank, or Conduit	2,310	LF
24	L-108-5.2	No. 6 AWG, Bare CU Counterpoise Installed in Trench	1,670	LF
25	L-110-5.1	Non-Encased Conduit, 1-Way 2"	1,190	LF
26	L-110-5.2	Concrete Encased Conduit, 1-Way 2"	170	LF
27	L-110-5.3	Concrete Encased Conduit, 2-Way 2"	90	LF
28	L-110-5.4	Directional Bored Duct Bank, 4-Way 2"	220	LF
29	L-115-5.1	Concrete H-20 Load Rated Electrical Handhole	3	EA
30	L-125-5.1	NEW L-861T Elevated Taxiway Edge Light Installed on L-867 Base Can including Isolation Transformer, stem, plate and splice kit	30	EA
31	L-125-5.2	NEW L-861T Elevated Taxiway Edge Light Installed on L-868(B) Base Can including Isolation Transformer, stem, plate and splice kit	1	EA
32	L-125-5.3	New 2 Module Size 1 LED Airfield Sign with Concrete Base	2	EA
33	L-125-5.4	New 3 Module Size 1 LED Airfield Sign with Concrete Base	2	EA
34	L-125-5.5	New 4 Module Size 1 LED Airfield Sign with Concrete Base	1	EA

**ALTERNATE 1 BID QUANTITIES**

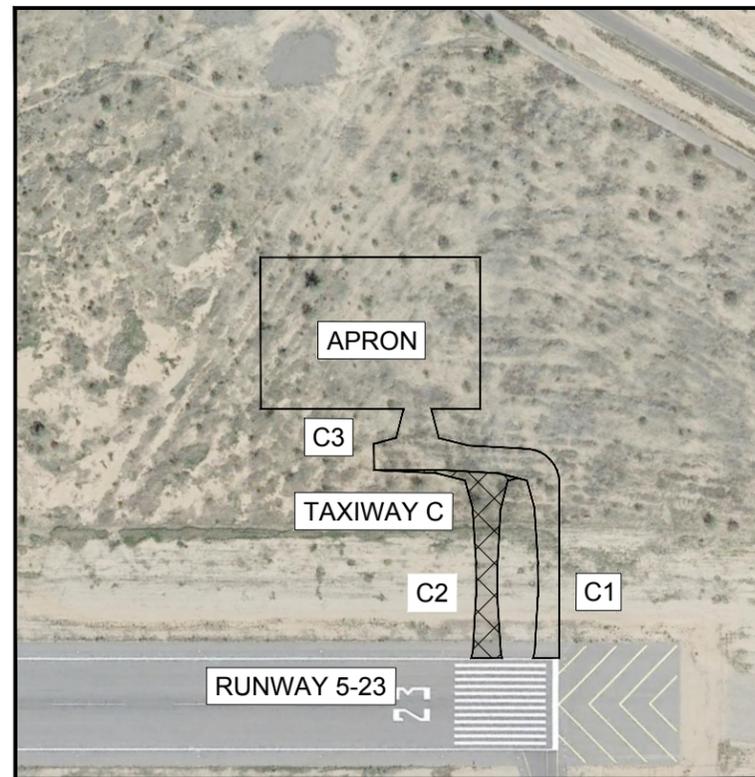
Item No.	Specification Number	Bid Item Description	Quantity	Unit
1	C-100	Contractor Quality Control	1	LS
2	C-102-5.1	Storm Water Pollution Prevention	1	LS
3	C-105	Mobilization/Demobilization (Maximum of 5%)	1	LS
4	SP-60.1	Airside Safety and Security / Traffic Control	1	LS
5	P-101-5.1	Bituminous Pavement Removal (4 In. Depth)	140	SY
6	P-151-4.1	Clearing and Grubbing	1	AC
7	P-152-4.1	Unclassified Excavation	290	CY
8	P-152-4.2	Fill with Select Material	270	CY
9	P-152-5.1	Compacted Subgrade (10 In. Depth)	1,790	SY
10	P-208.1	Aggregate Base Course (Stabilized Shoulder) (6")	140	CY
11	P-209.1	Crushed Aggregate Base Course (6")	300	CY
13	P-401-8.1	Asphalt Mix Pavement	400	TONS
15	P-620.1	Reflective Yellow Taxiway Pavement Markings, Waterborne	450	SF
16	P-620.2	Reflective White Runway Pavement Markings, Waterborne	180	SF
17	P-620.3	Black Taxiway Pavement Markings, Waterborne	410	SF
18	D-701-5.1	24-inch Concrete Pipe RGRCP, Class V	110	LF
19	D-751-5.2	Storm Drain Inlet	2	EA
20	D-751-5.3	Storm Drain Apron	2	EA
21	MAG 220.5	Ungrouted Rip Rap, D50=9", with Geosynthetic Fabric No. 8 AWG, 5 kV, L-824, Type C Cable, Installed in Trench, Duct Bank, or Conduit	50	CY
22	L-108-5.1	No. 8 AWG, 5 kV, L-824, Type C Cable, Installed in Trench, Duct Bank, or Conduit	1,275	LF
23	L-108-5.2	No. 6 AWG, Bare CU Counterpoise Installed in Trench	855	LF
24	L-110-5.1	Non-Encased Conduit, 1-Way 2"	750	LF
25	L-110-5.3	Concrete Encased Conduit, 2-Way 2"	105	LF
26	L-115-5.1	Concrete H-20 Load Rated Electrical Handhole	1	EA
27	L-125-5.1	NEW L-861T Elevated Taxiway Edge Light Installed on L-867 Base Can including Isolation Transformer, stem, plate and splice kit	18	EA
28	L-125-5.3	New 2 Module Size 1 LED Airfield Sign with Concrete Base	1	EA
29	L-125-5.5	New 4 Module Size 1 LED Airfield Sign with Concrete Base	1	EA

**ALTERNATE 2 BID QUANTITIES**

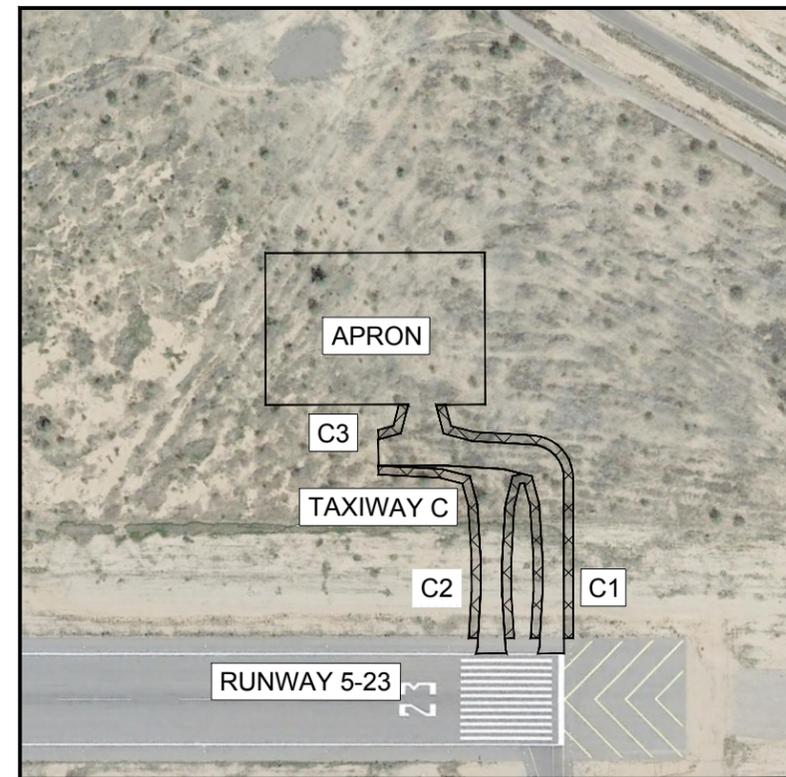
Item No.	Specification Number	Bid Item Description	Quantity	Unit
1	C-100	Contractor Quality Control	1	LS
2	C-102-5.1	Storm Water Pollution Prevention	1	LS
3	C-105	Mobilization/Demobilization (Maximum of 5%)	1	LS
4	SP-60.1	Airside Safety and Security / Traffic Control	1	LS
5	P-152-4.1	Unclassified Excavation	470	CY
6	P-152-4.2	Fill with Select Material	40	CY
7	P-152-5.1	Compacted Subgrade (10 In. Depth)	3,060	SY
8	MAG 702-4.1	Crushed Aggregate Base Course (6") (Not FAA AIP Eligible)	510	CY
9	MAG 710-5.1	Asphalt Concrete (4") (Not FAA AIP Eligible)	510	TONS
10	P-620.1	Reflective Yellow Taxiway Pavement Markings, Waterborne	1,740	SF
11	L-125-5.6	NEW L-861T Elevated Taxiway Edge Light Installed on L-867 Base Can with Base Can Extension including Isolation Transformer, stem, plate and splice kit (price differential from Taxiway Edge Lights specified per Base Bid and Alternate 1)	48	EA



**BASE BID**  
SCALE: 1"=150'



**ALTERNATIVE #1**  
SCALE: 1"=150'



**ALTERNATIVE #2**  
SCALE: 1"=150'

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5/20/25
DATE

ADDITIONAL 01
REVISION

NO.
NO.

**Kimley-Horn**  
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7740 North 16th Street, Suite 300  
Phoenix, Arizona 85020 (602) 944-5500

Professional Engineer (Civil)  
BRANDON ROBINSON  
No. 15120  
Expires 08/31/2028

SCALE (H): 1"=150'  
SCALE (V): N/A

DESIGNED BY: BLR  
DRAWN BY: SOR  
CHECKED BY: BLR

PROJECT NO.  
191593014

DRAWING NAME  
191593014T02.dwg

C1.3

SHEET NO. 3 OF 43

DATE: 04/30/25

COOLIDGE MUNICIPAL AIRPORT  
CITY OF COOLIDGE  
GROWTH MANAGEMENT  
131 W PINKLEY AVE  
COOLIDGE, AZ 85228

COOLIDGE MUNICIPAL AIRPORT  
GA APRON AND TAXIWAY C  
GENERAL NOTES

Call at least two full working days before your dig starts.  
ARIZONA 811  
Arizona's One-Call System  
Dial 8-1-1 or 1-800-874-8111 (782-8111)  
No Duplicates Allowed (902) 363-1100

**SURVEY CONTROL NOTES:**

THE FIELD PORTION OF THIS SURVEY WAS PERFORMED DURING THE MONTH OF JULY, 2024.

 **SURVEY CONTROL POINT**

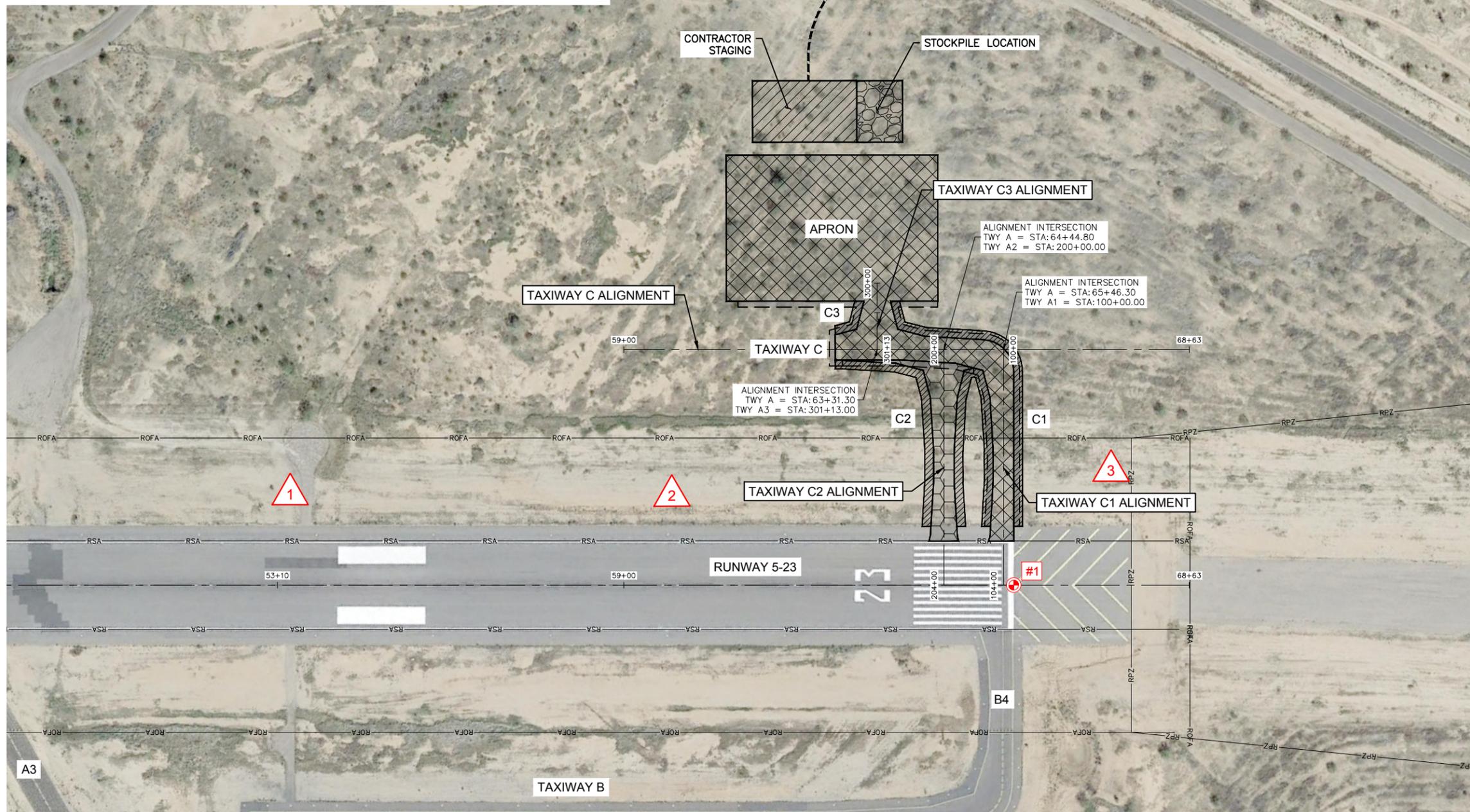
CONTROL POINT	NORTHING	EASTING	ELEVATION	DESCRIPTION
1	706762.99	852514.92	1575.19	SET 60D NAIL
2	706352.01	851888.71	1572.23	SET R+YC
3	706029.02	851324.77	1569.30	SET R+YC BM

 **PRIMARY CONTROL POINT**

CONTROL POINT	NORTHING	EASTING	ELEVATION	DESCRIPTION
1	706511.49	852468.60	1575.99	FOUND WOOLPERT 1" CAP 2009 BM

**ALIGNMENT DEFINITION TABLE**

ALIGNMENT	BEARING	START STA.	START NORTHING	START EASTING	END STA.	END NORTHING	END EASTING
TAXIWAY C	N59°58'20"E	59+00	706525.81	851693.79	68+63	707007.61	852527.34
TAXIWAY C1	S30°01'40"E	100+00	706849.24	852253.34	104+00	706502.92	852453.51
TAXIWAY C2	S30°01'40"E	200+00	706798.45	852165.47	204+00	706452.13	852365.64
TAXIWAY C3	S30°01'40"E	300+00	706839.48	852010.65	301+13	706741.65	852067.20



**LEGEND**

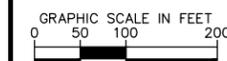
-  PROJECT LIMITS (BASE BID)
-  PROJECT LIMITS (ALTERNATE BID 1)
-  PROJECT LIMITS (ALTERNATE BID 2)
-  STOCKPILE AREA
-  CONTRACTOR'S STAGING AREA (CONTRACTOR TO COORDINATE ACTUAL LOCATION WITH AIRPORT PRIOR TO STARTING CONSTRUCTION)
-  PRIMARY CONTROL POINT
-  SURVEY CONTROL POINT
-  HAUL ROUTE / SITE ACCESS
-  RSA — RUNWAY SAFETY AREA
-  ROFA — RUNWAY OBJECT FREE AREA
-  RPZ — RUNWAY PROTECTION ZONE

**GENERAL NOTES:**

- THE CONTRACTOR SHALL COORDINATE ANY NECESSARY CLOSURES WITH THE AIRPORT, PRIOR TO STARTING WORK.
- THE CONTRACTOR SHALL VERIFY THE BARRICADES FOR EACH PHASE OF THE PROJECT ARE IN THE CORRECT LOCATION, ARE PROPERLY SECURED, AND ARE IN PROPER WORKING ORDER PRIOR TO STARTING WORK EACH DAY/NIGHT OF THE PROJECT.
- THE CONTRACTOR SHALL NOT STRAY FROM THE HAUL ROUTE / SITE ACCESS DESIGNATED ON THE PROJECT LAYOUT PLAN.
- THE CONTRACTOR IS RESPONSIBLE FOR MAINTAINING THE HAUL ROUTE / SITE ACCESS DURING THE PROJECT. THE CONTRACTOR WILL REPAIR THE HAUL ROUTE / SITE ACCESS AS NECESSARY AT THE END OF THE PROJECT (AT THE CONTRACTOR'S EXPENSE).
- CONTRACTOR SHALL COORDINATE WITH AND OBTAIN APPROVAL FROM AIRPORT OPERATIONS REGARDING USE OF ANY AIRPORT GATE. AIRPORT OPERATIONS MAY ALLOW ACCESS FROM OTHER GATES BASED ON AVAILABILITY OF SECURITY STAFF.
- CONTRACTOR TO ENSURE THAT NO CONSTRUCTION TRAFFIC ENTERING SITE BACKS UP ON OR BLOCKS ANY PUBLIC STREET.
- ANY EXISTING PAVEMENT AREA ALONG ANY HAUL ROUTE SHALL BE ADEQUATELY PLATED, UNLESS APPROVED OTHERWISE PRIOR TO HAULING ACTIVITIES.

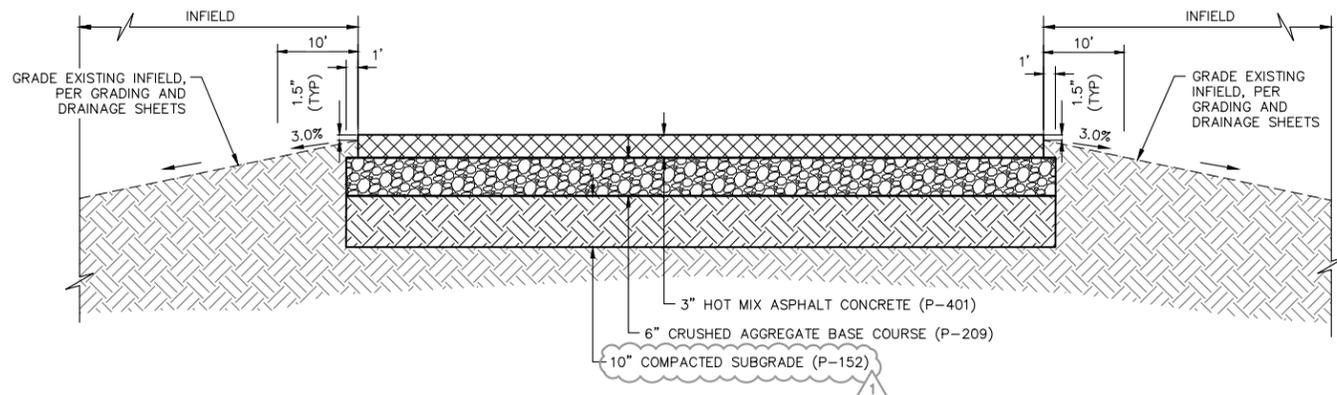
**PROJECT DESCRIPTION**

THIS PROJECT CONSISTS OF CONSTRUCTING A GENERAL AVIATION APRON WITH A PARTIAL PARALLEL TAXIWAY C AND CONNECTOR TAXIWAYS C1, C2 AND C3 AT THE COOLIDGE MUNICIPAL AIRPORT IN COOLIDGE, AZ. THIS PROJECT WILL INCLUDE THE SAWCUTTING AND REMOVAL OF EXISTING AC PAVEMENT, EXCAVATION AND INSTALLATION OF DRAINAGE AND ELECTRICAL INFRASTRUCTURE, GRADING, AGGREGATE BASE COURSE, CONSTRUCTION OF NEW AC PAVEMENT AND PAVEMENT MARKINGS.

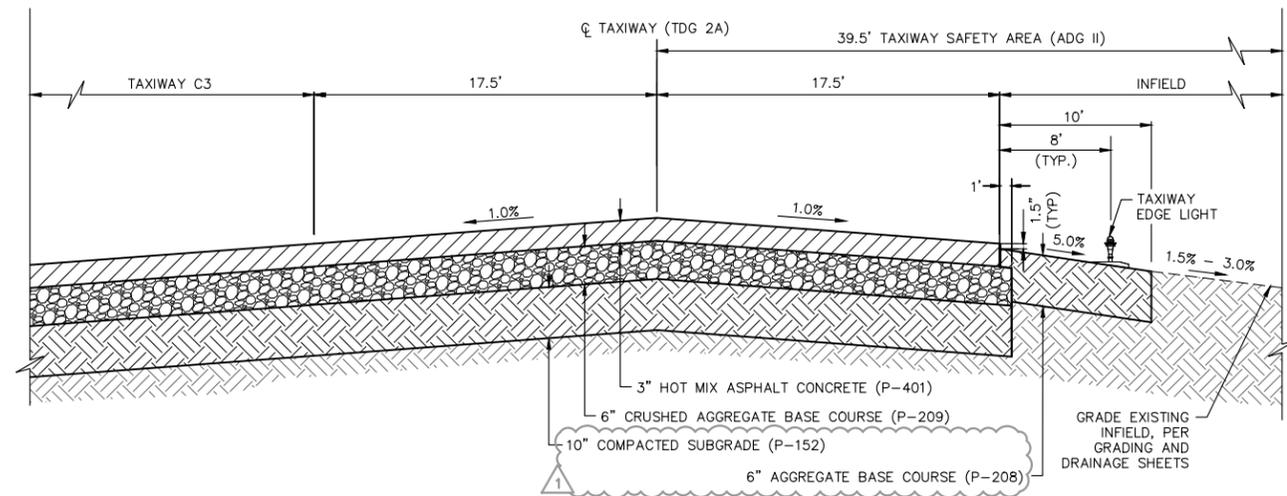


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<p>SCALE (H): 1" = 100' SCALE (V): N/A</p> <p>DESIGNED BY: BLR DRAWN BY: SQR CHECKED BY: BLR</p> <p>DATE: 04/30/25</p>	<p>COOLIDGE MUNICIPAL AIRPORT CITY OF COOLIDGE GROWTH MANAGEMENT 131 W PINKLEY AVE COOLIDGE, AZ 85228</p>
<p>COOLIDGE MUNICIPAL AIRPORT GA APRON AND TAXIWAY C PROJECT LAYOUT</p>	
<p>PROJECT NO. 191593014</p> <p>DRAWING NAME 191593014PL.dwg</p> <p>C1.4</p> <p>SHEET NO. 4 OF 43</p>	

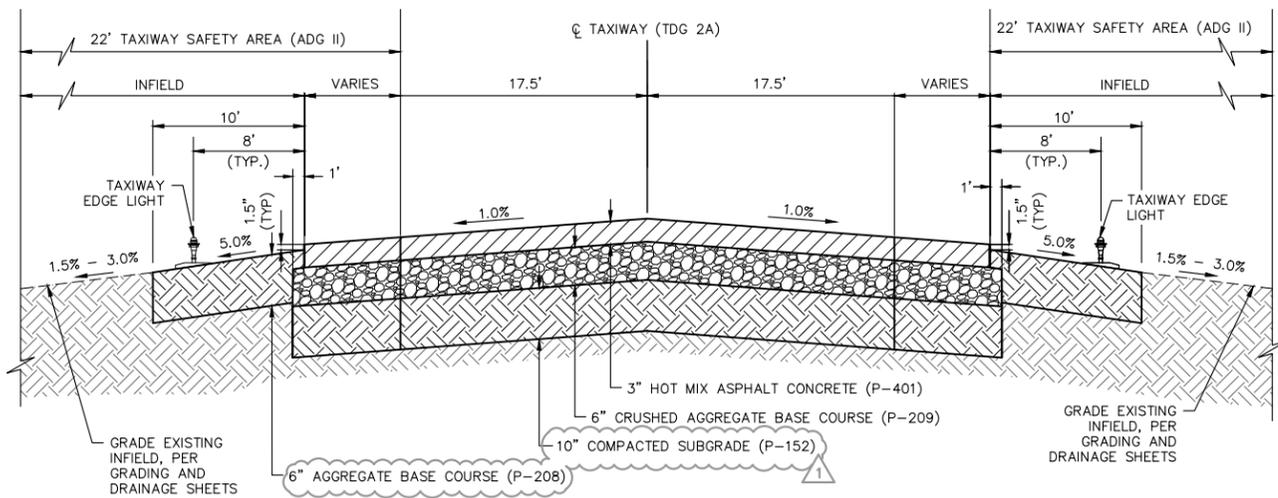
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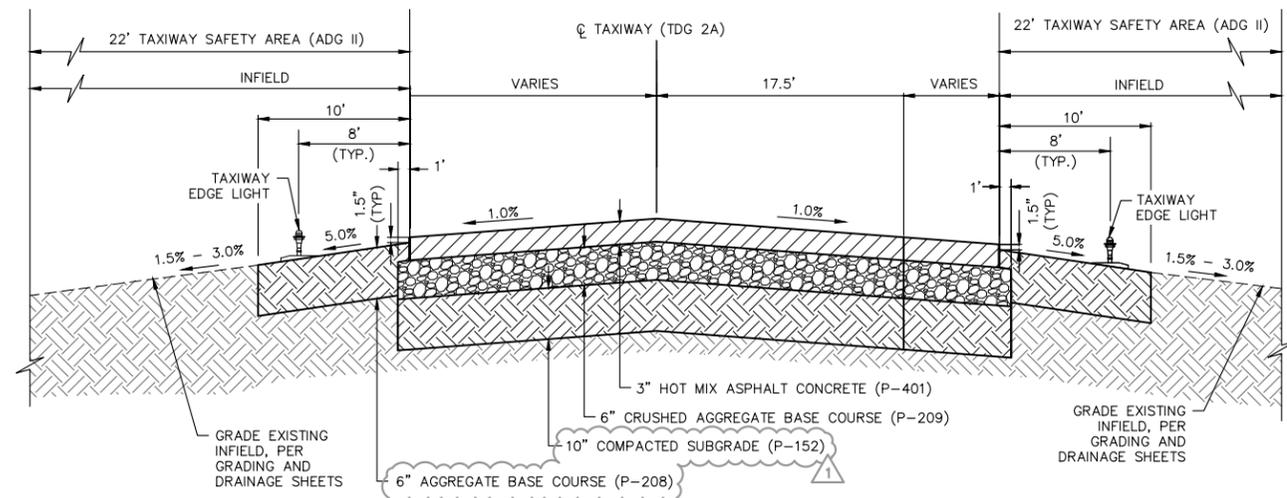
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N.T.S.



3  
C3.1 TAXIWAY C TYPICAL SECTION  
(TAXIWAY C ALIGNMENT - STA: 63+74.30 TO STA: 65+03.30)  
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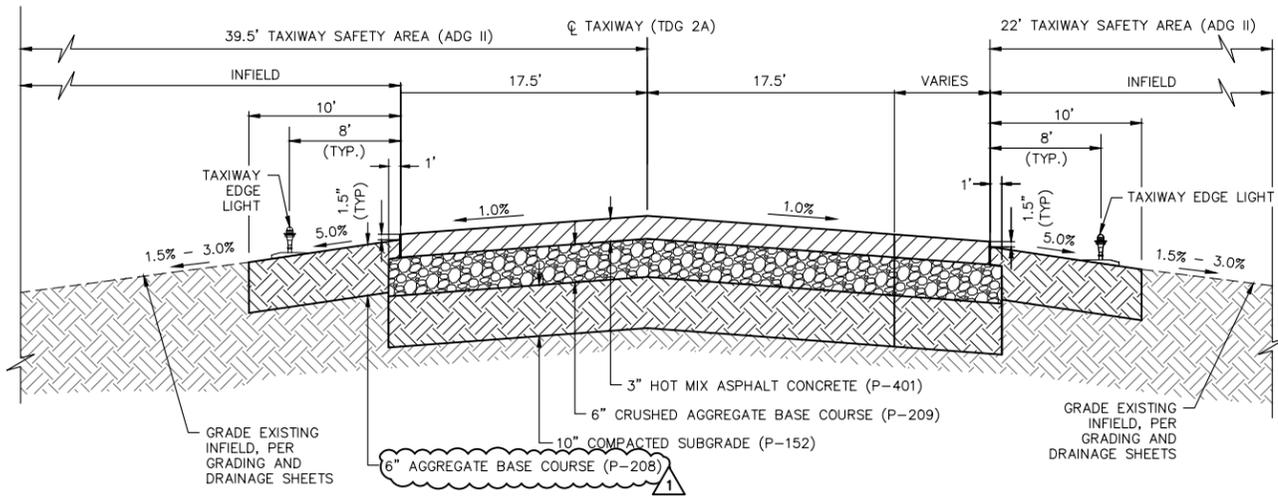


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C3.1 TAXIWAY C TYPICAL SECTION  
(TAXIWAY C ALIGNMENT - STA: 65+03.30 TO STA: 65+49.32)  
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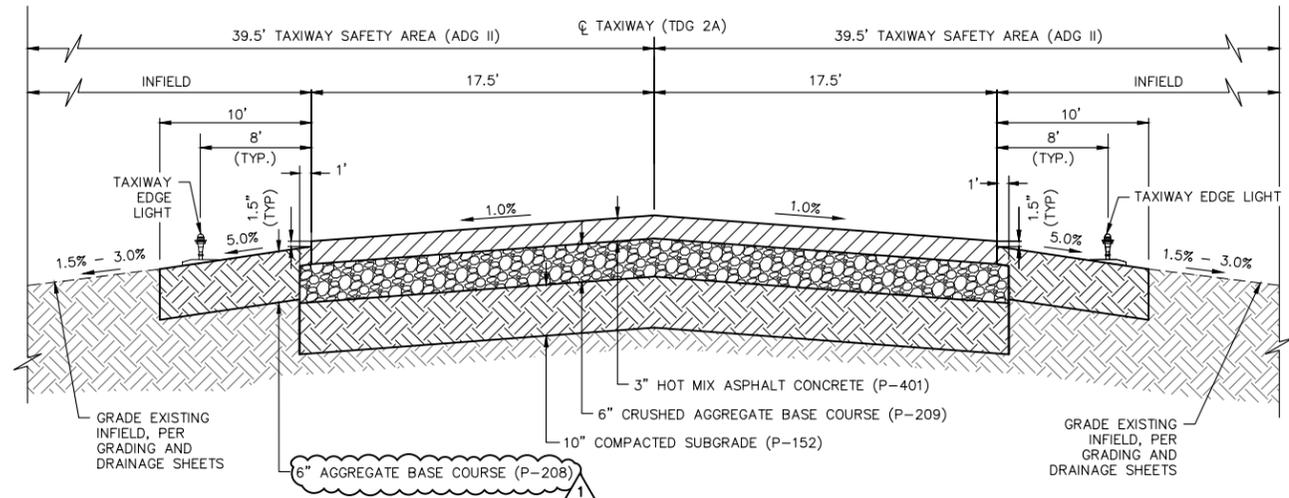


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COOLIDGE MUNICIPAL AIRPORT CITY OF COOLIDGE GROWTH MANAGEMENT 131 W PINKLEY AVE COOLIDGE, AZ 85228	
COOLIDGE MUNICIPAL AIRPORT GA APRON AND TAXIWAY C TYPICAL SECTIONS (BASE BID)	
PROJECT NO. 191593014	DRAWING NAME 191593014TS.dwg
C3.1 SHEET NO. 9 OF 43	



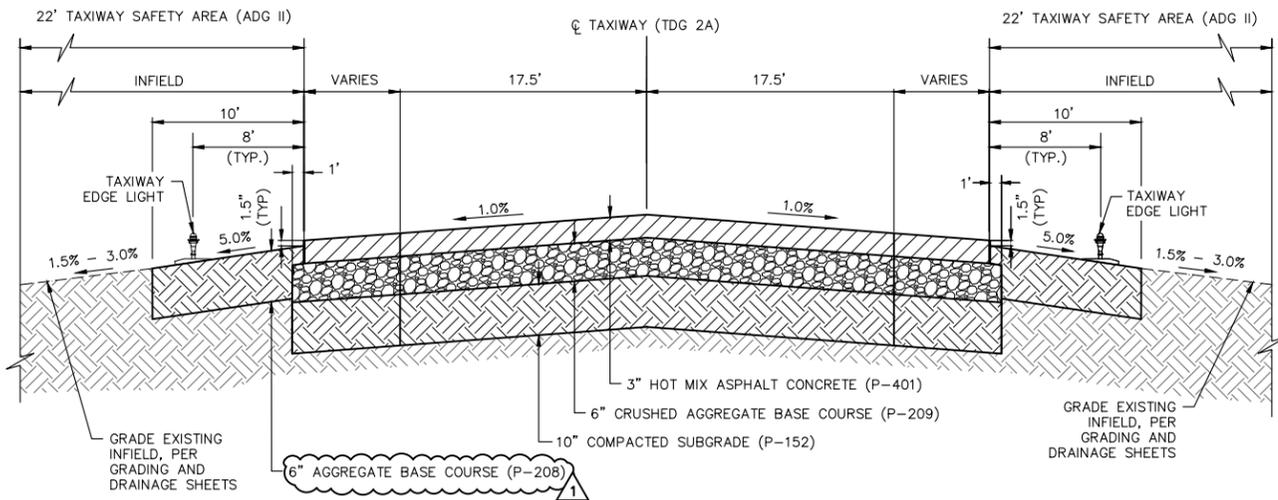
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 AND STA: 102+25.00 TO 103+25.00)  
 N.T.S.

5  
C3.2



**TAXIWAY C1 TYPICAL SECTION**  
 (TAXIWAY C1 ALIGNMENT - STA: 101+72.00 TO STA: 102+25.00)  
 N.T.S.

6  
C3.2



**TAXIWAY C3 TYPICAL SECTION**  
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7  
C3.2

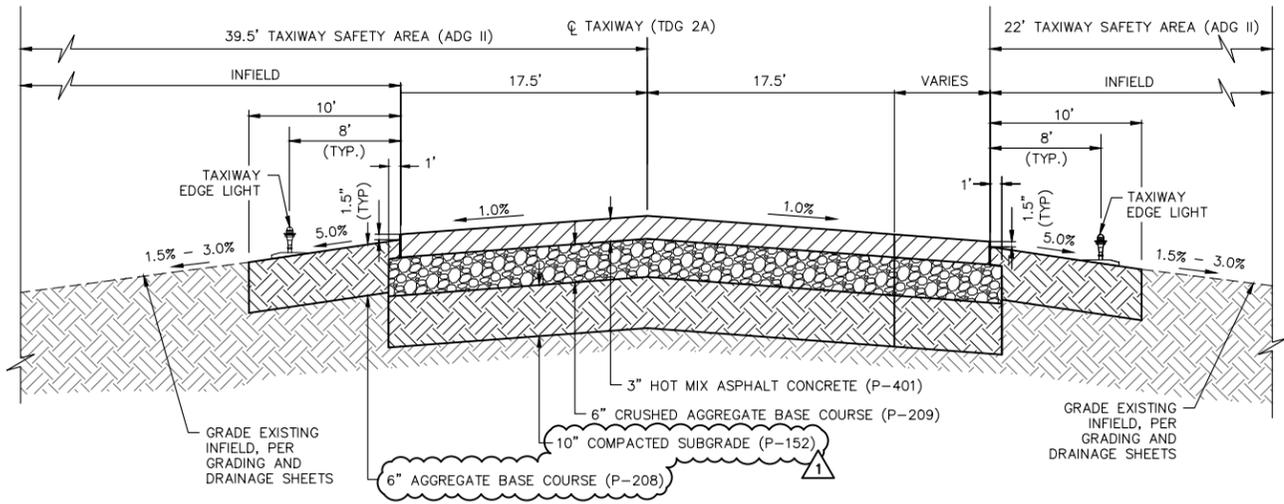
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COOLIDGE MUNICIPAL AIRPORT CITY OF COOLIDGE GROWTH MANAGEMENT 131 W PINKLEY AVE COOLIDGE, AZ 85228	
COOLIDGE MUNICIPAL AIRPORT GA APRON AND TAXIWAY C TYPICAL SECTIONS (BASE BID)	
PROJECT NO. 191593014	DRAWING NAME 191593014TS_2.dwg
C3.2 SHEET NO. 10 OF 43	



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1  
 C3.3 TAXIWAY C2 TYPICAL SECTION  
 N.T.S.

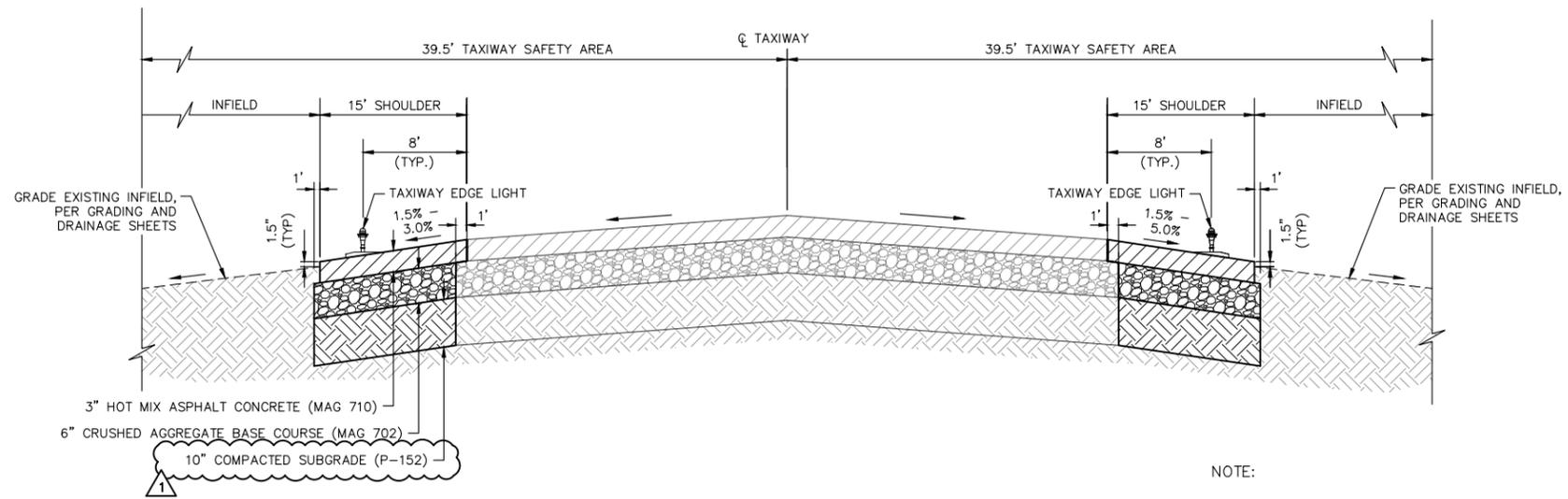


PROJECT NO. 191593014				<b>COOLIDGE MUNICIPAL AIRPORT</b> CITY OF COOLIDGE GROWTH MANAGEMENT 131 W PINKLEY AVE COOLIDGE, AZ 85228	<b>Kimley»Horn</b> © 2025 KIMLEY-HORN AND ASSOCIATES, INC. 7740 North 16th Street, Suite 300 Phoenix, Arizona 85020 (602) 944-5500	ADDENDUM 01	REVISION	5/30/25	DATE
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COOLIDGE MUNICIPAL AIRPORT  
 GA APRON AND TAXIWAY C  
 TYPICAL SECTIONS  
 (ALTERNATIVE 1)

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 DRAWN BY: SQR  
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**1**  
**C3.4** TAXIWAY SHOULDER TYPICAL SECTION  
 N.T.S.

NOTE:  
 PROPOSED PAVED TAXIWAY  
 SHOULDERS EXCEED FAA  
 STANDARDS AND ARE NOT  
 ELIGIBLE FOR FAA FUNDING.



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 CITY OF COOLIDGE  
 GROWTH MANAGEMENT  
 131 W PINKLEY AVE  
 COOLIDGE, AZ 85228

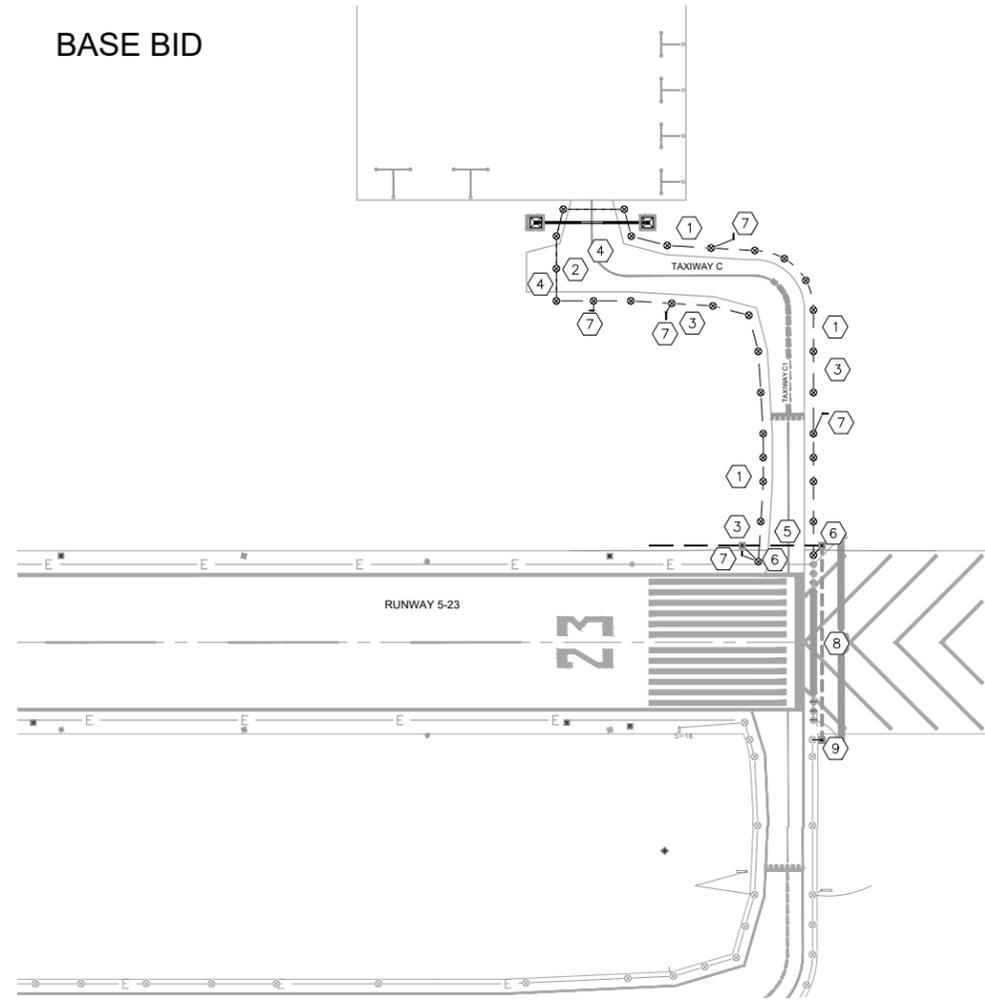
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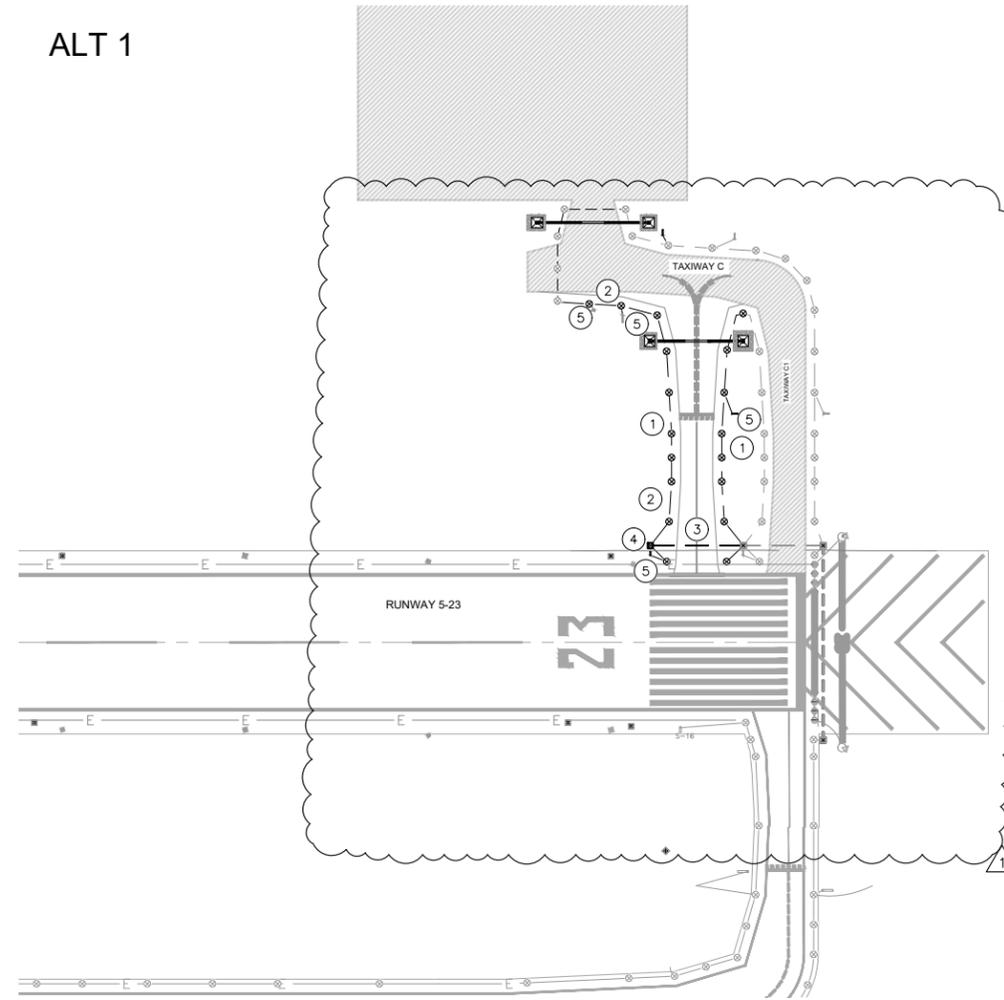
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BASE BID



ALT 1



ABBREVIATIONS

ALV	AIRFIELD LIGHTING VAULT
ATCT	AIR TRAFFIC CONTROL TOWER
A.T.S.	AUTOMATIC TRANSFER SWITCH
AWOS	AUTOMATED WEATHER OBSERVATION SYSTEM
C, CNDT	CONDUIT
CCR	CONSTANT CURRENT REGULATOR
C.E.	CONCRETE ENCASED
CLSM	CONTROLLED LOW STRENGTH MATERIAL
D.B.	DIRECT EARTH BURIAL
(E)	EXISTING EQUIPMENT OR FACILITY
FAA	FEDERAL AVIATION ADMINISTRATION
HH	HANDHOLE
HIRL	HIGH INTENSITY RUNWAY LIGHTS
ILS	INSTRUMENT LANDING SYSTEM
MALSR	MEDIUM INTENSITY APPROACH LIGHTING SYSTEM WITH ALIGNMENT INDICATOR LIGHTS
RUNWAY	RUNWAY
(L)	LED (LIGHT EMITTING DIODE)
(N)	NEW EQUIPMENT OR FACILITY
PAPI	PRECISION APPROACH PATH INDICATOR
P.O.C.	POINT OF CONNECTION (WITH EXISTING CONDUIT OR DUCT)
PVC	POLYVINYLCHLORIDE: CONDUIT MATERIAL
(R)	REMOVE EQUIPMENT OR FACILITY
(RR)	REMOVE AND REPLACE EQUIPMENT OR FACILITY
RDR	RUNWAY DISTANCE REMAINING
REIL	RUNWAY END INDICATOR LIGHT
RSA	RUNWAY SAFETY AREA
R/W, RWY	RUNWAY
SCH	SCHEDULE
TSA	TAXIWAY SAFETY AREA
T/W, TWY	TAXIWAY
TYP.	TYPICAL
U.O.N.	UNLESS OTHERWISE NOTED

LEGEND

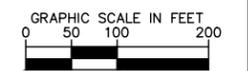
	EXISTING CONDUIT OR DUCT BANK TO REMAIN
	PROPOSED 1-2" SCH. 40 PVC DIRECT BURIED CONDUIT
	PROPOSED 1-2" SCH. 40 PVC CONCRETE ENCASED CONDUIT
	PROPOSED 2-2" SCH. 40 PVC CONCRETE ENCASED CONDUIT
	PROPOSED 4-2" SCH. 80 HDPE DIRECTIONAL BORE
	EXISTING HANDHOLE TO REMAIN, PROTECT IN PLACE
	EXISTING RUNWAY EDGE/END LIGHT TO REMAIN
	EXISTING L-861T TAXIWAY EDGE LIGHT TO REMAIN
	EXISTING AIRFIELD GUIDANCE SIGN TO REMAIN
	EXISTING REIL TO REMAIN
	PROPOSED L-861T TAXIWAY EDGE LIGHT
	PROPOSED AIRFIELD GUIDANCE SIGN ON NEW BASE
	PROPOSED H-20 LOAD RATED HANDHOLE

ELECTRICAL SCOPE OF WORK- BASE BID

- 1 INSTALL NEW L-861(T) LED ELEVATED TAXIWAY EDGE LIGHT, ISOLATION TRANSFORMER, AND L-824 #8 5KV AIRFIELD CABLING ON NEW L-867 BASE CAN
- 2 INSTALL NEW L-861(T) LED ELEVATED TAXIWAY EDGE LIGHT, ISOLATION TRANSFORMER, AND L-824 #8 5KV AIRFIELD CABLING ON NEW L-868(B) BASE CAN
- 3 INSTALL NEW 1-2" SCH 40 PVC DIRECT BURIED CONDUIT AND L-824 #8 5KV AIRFIELD CABLING.
- 4 INSTALL NEW 1-2" SCH 40 PVC CONCRETE ENCASED CONDUIT AND L-824 #8 5KV AIRFIELD CABLING.
- 5 INSTALL NEW 2-2" SCH 40 PVC CONCRETE ENCASED CONDUIT AND L-824 #8 5KV AIRFIELD CABLING.
- 6 INSTALL NEW AIRCRAFT LOAD RATED CONCRETE HANDHOLE.
- 7 INSTALL NEW L-858(L) LED, SIZE 1, AIRFIELD GUIDANCE SIGN AND ISOLATION TRANSFORMER ON NEW CONCRETE FOUNDATION.
- 8 INSTALL NEW 4-2" SCH 80 HDPE DIRECTIONAL BORE AND L-824 #8 5KV AIRFIELD CABLING.
- 9 TIE IN TO EXISTING TAXIWAY B EDGE LIGHT CIRCUIT AT NEW HANDHOLE.

ELECTRICAL SCOPE OF WORK- ALT 1

- 1 INSTALL NEW L-861(T) LED ELEVATED TAXIWAY EDGE LIGHT, ISOLATION TRANSFORMER, AND L-824 #8 5KV AIRFIELD CABLING ON NEW L-867 BASE CAN
- 2 INSTALL NEW 1-2" SCH 40 PVC DIRECT BURIED CONDUIT AND L-824 #8 5KV AIRFIELD CABLING.
- 3 INSTALL NEW 2-2" SCH 40 PVC CONCRETE ENCASED CONDUIT AND L-824 #8 5KV AIRFIELD CABLING.
- 4 INSTALL NEW AIRCRAFT LOAD RATED CONCRETE HANDHOLE.
- 5 INSTALL NEW L-858(L) LED, SIZE 1, AIRFIELD GUIDANCE SIGN AND ISOLATION TRANSFORMER ON NEW CONCRETE FOUNDATION.



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 Plot Device: HP DesignJet T1100e  
 Plot Style: HP-Plot-Style.ctb  
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COOLIDGE MUNICIPAL AIRPORT GA APRON AND TAXIWAY C ELECTRICAL SCOPE OF WORK (BASE BID)	
PROJECT NO. 191593014 DRAWING NAME 191593014ESOW.dwg E1.0 SHEET NO. 20 OF 43	









**ATTACHMENT IV**  
**GEO TECHNICAL REPORT**



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# GEOTECHNICAL EVALUATION REPORT

**COOLIDGE MUNICIPAL AIRPORT  
GENERAL AVIATION APRON AND TAXIWAY C**  
6300 North Coolidge Airport Road  
Coolidge, Arizona  
WT Job No. 29-224083-0, Revision 1

**PREPARED FOR:**  
Kimley-Horn and Associates, Inc.  
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Mesa, Arizona 85210  
Attn: Brandon Robinson, P.E.

November 19, 2024

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GEOTECHNICAL    ENVIRONMENTAL    INSPECTIONS    NDT    MATERIALS

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**GEOTECHNICAL EVALUATION  
COOLIDGE MUNICIPAL AIRPORT GA APRON AND TAXIWAY C  
6300 NORTH COOLIDGE AIRPORT ROAD  
COOLIDGE, ARIZONA  
JOB NO. 29-224083-0  
REVISION 1**

**1.0 PURPOSE**

This report contains the results of our geotechnical evaluation for a proposed apron and taxiway to be located in Coolidge, Arizona. The purpose of these services is to provide information and recommendations regarding:

- Subsurface conditions
- Earthwork guidelines
- Pavement sections
- Drainage
- Groundwater
- Corrosivity (soil to concrete)
- Seismic conditions
- Excavation conditions

Results of the field exploration, field tests, and laboratory testing program are presented in the Appendices.

**2.0 PROJECT DESCRIPTION**

The project consists of the construction of an approximately 400-foot by 200-foot apron and approximately 1,000 feet of taxiway. We anticipate that final grades will be within 2 feet of the existing grade. Should this information not be correct, we should be notified immediately.

**3.0 SCOPE OF SERVICES**

**3.1 Field Exploration**

Eleven borings were drilled to depths ranging from about 8.5 to 11 feet below existing site grade in the proposed apron and taxiway areas. The borings were at the approximate locations shown on the attached Boring Location Diagram. A field log was prepared for each boring. These logs contain visual classifications of the materials encountered during drilling

as well as interpolation of the subsurface conditions between samples. Final logs, included in Appendix A, represent our interpretation of the field logs and may include modifications based on laboratory observations and tests of the field samples. The final logs describe the materials encountered, their thickness, and the locations where samples were obtained.

The Unified Soil Classification System was used to classify soils. The soil classification symbols appear on the boring logs and are briefly described in Appendix A. Local and regional geologic characteristics were used to estimate the seismic design criteria.

### **3.2 Laboratory Analyses**

Laboratory analyses were performed on representative soil samples to aid in material classification and to estimate pertinent engineering properties of the on-site soils for preparation of this report. Testing was performed in general accordance with applicable standard test methods. The following tests were performed, and the results are presented in Appendix B.

- Water content
- Dry density
- Compression
- Expansion
- Plasticity
- Minus #200 sieve
- Soil pH
- Minimum electrical resistivity
- California bearing ratio (CBR)
- Moisture-density relationship (proctor)
- Soluble sulfate and chloride content

### **3.3 Analyses and Report**

This geotechnical engineering report includes a description of the project, a discussion of the field and laboratory testing programs, a discussion of the subsurface conditions, and design recommendations as appropriate to its purpose. The scope of services for this project does not include, either specifically or by implication, any environmental assessment of the site, discovery of underground storage tanks or other underground structures, or identification of contaminated or hazardous materials or conditions. If there is concern about the potential for such contamination, other studies should be undertaken. We are available to discuss the scope of such studies with you.

## 4.0 SITE CONDITIONS

### 4.1 Surface

At the time of our exploration, the site was an undeveloped area of Coolidge Municipal Airport on the north side of Runway 5-23 near its east end. The ground surface was relatively flat and contained a sparse to moderate growth of trees, brush, and grasses. Site drainage trended to the northeast as sheet surface flow. The site is bound by Runway 5-23 to the southeast, undeveloped land to the southwest, and undeveloped land followed by North Coolidge Airport Road to the northeast and northwest.

### 4.2 Subsurface

As presented on the Boring Logs, surface soils to the full depth of exploration consisted of loose to very dense Clayey SAND. Near surface soils are of medium plasticity. No apparent zones of carbonate cementation were encountered. Groundwater was not encountered in any boring at the time of exploration. A detailed description of the soils encountered can be found on the boring logs in Appendix A.

## 5.0 GEOTECHNICAL PROPERTIES & ANALYSIS

### 5.1 Laboratory Tests

Laboratory test results (see Appendix B) indicate that near-surface soils exhibit low compressibility at existing water contents. Low to high additional compression occurs when the water content is increased.

Near-surface soils are of medium plasticity. These soils exhibit low expansion potential when recompacted, confined by loads approximating pavement loads and saturated. Slabs-on-grade supported on recompacted on-site soils have a low potential for heaving if the water content of the soil increases.

Chemical tests were performed on representative samples of on-site soils to determine the amount of water-soluble sulfates and chlorides. The test results indicate that the soils classify as negligibly corrosive to concrete according to Table 19.3.1.1 of ACI 318-19. The tests were performed by Motzz Laboratories, Inc. and the test results are presented in Appendix B.

Minimum electrical resistivity and hydrogen ion concentration (pH) were performed on representative samples to aid in assessing, by others, the potential for corrosion of buried metals. The test results are presented in Appendix B.

## **5.2 Field Tests**

Existing subsoils near shallow foundation level exhibited low to moderate resistance to penetration using test method ASTM D3550. This corresponds to a moderate bearing capacity for existing soils in their present condition. However, penetration resistance values exhibited some variability between test locations. This represents a potential for differential settlements within structures supported on the existing soils in their present condition.

## **6.0 RECOMMENDATIONS**

### **6.1 General**

Recommendations contained in this report are based on our understanding of the project criteria described in Section 2.0 and the assumption that the soil and subsurface conditions are those disclosed by the explorations. Others may change the plans, final elevations, number and type of structures, foundation loads, and floor levels during design or construction. Substantially different subsurface conditions from those described herein may be encountered or become known. Any changes in the project criteria or subsurface conditions shall be brought to our attention in writing. This report does not encompass the effects, if any, of underlying geologic hazards or regional groundwater withdrawal and expresses no opinion regarding their effects on surface movements at the project site.

### **6.2 Apron and Taxiway Pavements**

Apron and taxiway pavements are being designed to accommodate a mix of commercial and general aviation aircraft (see Tables 1 and 2, below). FAA pavement design software (FAARFIELD) has an internal library of aircraft, but does not include all of the aircraft in the table. Therefore, substitutions are noted.

**Table 1: Design Fleet Mix A**

Aircraft	Weight (pounds)	Annual Departures <sup>1</sup>
Lockheed C-130	155,000	240
Transall C-160 <sup>2</sup>	112,435	80
Shorts C-23 Sherpa <sup>3</sup>	22,900	2,448
General Aviation	12,500	6,000

**Table 2: Design Fleet Mix B**

Aircraft	Weight (pounds)	Annual Departures <sup>1</sup>
Shorts C-23 Sherpa <sup>3</sup>	22,900	2,448
General Aviation	12,500	6,000

Recommended pavement sections presented below were designed in accordance with FAA (Federal Aviation Administration) advisory circular AC 150-5320-6G. Pavement designs were calculated using the FAARFIELD computer program (v2.1.1) associated with this FAA advisory circular. A design CBR of 22 was used for untreated site soils.

The following pavement sections were determined to be applicable to the project:

**Table 3: Alternate Pavement Sections for Aprons and Taxiways – Design Fleet Mix A**

Option	P-401 Hot Mix Asphalt Concrete (inches)	P-304 Cement-Treated Base Course (inches)	P-209 Crushed Aggregate Base Course (inches)	P-152 Subgrade (Inches)
<b>1A</b>	4	5	--	10
<b>2A</b>	4	--	6	10

<sup>1</sup> An annual growth rate of 1.0 percent was used for all aircraft.

<sup>2</sup> Modeled in FAARFIELD as a “C-130” aircraft at the stated weight.

<sup>3</sup> Modeled in FAARFIELD as a “Shorts 330-200” aircraft at the stated weight.

**Table 4: Alternate Pavement Sections for Aprons and Taxiways – Design Fleet Mix B**

Option	P-401 Hot Mix Asphalt Concrete (inches)	P-304 Cement-Treated Base Course (inches)	P-208 Aggregate Base Course (inches)	P-152 Subgrade (Inches)
<b>1B</b>	3	4	--	10
<b>2B</b>	3	--	6	10

The "design life" (20 years) of a pavement is defined as the expected life at the end of which reconstruction of the pavement will need to occur. Normal maintenance, including crack sealing, slurry sealing, and/or chip sealing, should be performed during the life of the pavement.

Material and compaction requirements should conform to recommendations presented under **EARTHWORK**. The gradient of paved surfaces should ensure positive drainage. Water should not pond in areas directly adjoining paved sections.

After removing the existing pavements and excavating to the final subgrade elevation, the exposed subgrade should be proof-rolled to identify any zones of loose/soft or unstable soil. Proof-rolling may be accomplished with a loaded water truck or dump truck. Areas where soil movement is observed more than 6 inches away from the truck's rear tires should be considered unstable. In general, loose/soft or unstable soils should be removed to their full depth and replaced with properly compacted, engineered fill (Item P-152). Alternative methods to mitigate loose/soft or unstable soils may be appropriate depending upon the soil conditions observed at the time of construction. General alternatives that have been used successfully on previous projects are presented Section **7.7, Wet Subgrade Soils**, below.

### **6.3 Pavements For Shoulder Areas**

Shoulder sections will support emergency and maintenance equipment and 15 fully loaded passes of the most demanding aircraft. Shoulder and blast pad pavement thickness design was performed in accordance with Chapter 6 in AC 150/5320-6G. Evaluations of the design fleet mix aircraft revealed that the most demanding aircraft in the mix is the Lockheed C-130 for Design Fleet Mix A and Shorts C-23 Sherpa for Design Fleet Mix B. Based on 15

operations of these aircraft, the recommended sections presented in Table 5, below, were calculated using FAARFIELD.

**Table 5: Shoulder Pavement Sections**

Option	Design Fleet Mix	MAG <sup>4</sup> 710 Asphalt Concrete (inches)	P-304 Cement-Treated Base Course (inches)	MAG <sup>4</sup> 702 Crushed Aggregate Base (inches)	P-152 Subgrade (Inches)
1	A	4	5	--	10
2	A	4	--	6	10
3	B	3	4	--	10
4	B	3	--	6	10

Material and compaction requirements should conform to recommendations presented under **EARTHWORK**. The gradient of paved surfaces should ensure positive drainage. Water should not pond in areas directly adjoining paved sections.

After removing the existing pavements and excavating to the final subgrade elevation, the exposed subgrade should be proof-rolled to identify any zones of loose/soft or unstable soil. Proof-rolling may be accomplished with a loaded water truck or dump truck. Areas where soil movement is observed more than 6 inches away from the truck’s rear tires should be considered unstable. In general, loose/soft or unstable soils should be removed to their full depth and replaced with properly compacted, engineered fill (Item P-152). Alternative methods to mitigate loose/soft or unstable soils may be appropriate depending upon the soil conditions observed at the time of construction. General alternatives that have been used successfully on previous projects are presented Section **7.7, Wet Subgrade Soils**, below.

#### 6.4 Drainage

The major cause of soil-related foundation and slab-on-ground problems is moisture increase in soils below structures. Properly functioning conventional foundations and floor slabs-on-ground require appropriately constructed and maintained site drainage

---

<sup>4</sup> *Maricopa Association of Governments Uniform Standard Specifications for Public Works Construction*, latest edition. Local bituminous and aggregate mixes may be utilized in shoulder areas in lieu of FAA mixes P-401/403 and P208/209.

conditions. Therefore, it is extremely important that positive drainage be provided during construction and maintained throughout the life of each structure. It is also important that proper planning and control of landscape and irrigation practices be performed.

## **6.5 Corrosivity to Concrete**

The chemical test results indicate that the soils at the site classify as Class S0 in accordance with Table 19.3.1.1 of ACI 318-19. However, in order to be consistent with standard local practice and for reasons of material availability, we recommend that Type II Portland cement be used for all concrete on and below grade.

## **7.0 EARTHWORK**

### **7.1 General**

The conclusions contained in this report for the proposed construction are contingent upon compliance with recommendations presented in this section. Any excavating, trenching, or disturbance that occurs after completion of the earthwork must be backfilled, compacted and tested in accordance with the recommendations contained herein. It is not reasonable to rely upon our conclusions and recommendations if any future unobserved and untested trenching, earthwork activities or backfilling occurs.

Although fills or underground facilities such as septic tanks, cesspools, basements, utilities, and dry wells were not observed, such features might be encountered during construction. These features should be demolished in accordance with the recommendations of the geotechnical engineer. Any loose or disturbed soils resulting from demolition should be removed or recompacted as engineered fill and any excavations should be backfilled in accordance with recommendations presented herein.

### **7.2 Site Clearing**

Strip and remove any existing fill material, vegetation, debris, and any other deleterious materials from the pavement areas. All exposed surfaces should be free of mounds and depressions that could prevent uniform compaction.

### **7.3 Excavation**

We anticipate that excavations for shallow utility trenches for the proposed construction can be accomplished with conventional equipment.

On-site soils clayey soils may pump or become unworkable at high water contents. Workability may be improved by scarifying and drying. Over-excavation of wet zones and replacement with granular materials may be necessary. The use of lightweight excavation and compaction equipment may be required to minimize subgrade pumping.

Our soil classifications are based solely on the materials encountered in widely spaced exploratory test borings. The contractor should verify that similar conditions exist throughout the proposed area of excavation. If different subsurface conditions are found at the time of construction, we should be contacted immediately to evaluate the conditions encountered.

#### **7.3.1 Temporary Excavations and Slopes**

Temporary, non-surcharged construction excavations should be sloped or shored. The individual contractor should be made responsible for designing and constructing stable, temporary excavations as required to maintain stability of both the excavation sides and bottom. All excavations should be sloped or shored in the interest of safety following local and federal regulations, including current OSHA excavation and trench safety standards. OSHA recommends a maximum slope inclination of  $\frac{3}{4}$ :1 (horizontal:vertical) for Type A soils, 1:1 for Type B soils, and  $1\frac{1}{2}$ :1 for Type C soils.

As a safety measure, it is recommended that all vehicles and soil piles be kept a minimum lateral distance back from the crest of the slope at least equal to the slope height. The exposed slope face should be protected against the elements.

### **7.4 Pavement Preparation**

Earthwork to be performed for pavement support should comply with the requirements of Item P-152 "Excavation and Embankment" of FAA AC No. 150/5370-10G (or the latest edition available at the time of construction document preparation).

Where finished subgrade elevation is higher than existing site grade, existing soils should be scarified, moistened as required, and recompactd for a minimum depth of 8 inches prior to placement of fill.

## 7.5 **Earthwork and Pavement Materials**

Pavement subgrade and any fill required to obtain finished subgrade elevation, should be uniformly moisturized and compacted to at least 95 percent of the ASTM D 1557 dry-density value. Soils should be compacted within a water content range of 1 percent below to 3 percent above optimum and compacted to 95 percent of the maximum density as determined by ASTM D 1557. Uncompactd lift thicknesses should not to exceed 10 inches. Clean, on-site native soils or imported materials of equal or better pavement support quality may be used as fill material for the pavement areas.

Specifications for use during construction of the taxiway and apron would include:

- Item P-101, Surface Preparation
- Item P-151, Clearing and Grubbing
- Item P-152, Excavation, Subgrade, and Embankment
- Item P-155, Lime Treated Subgrade
- Item P-208, Aggregate Base Course
- Item P-209, Crushed Aggregate Base Course
- Item P-304, Cement-Treated Base Course
- Item P-401, Hot Mix Asphalt (HMA) Pavements
- Item P-403, Hot Mix Asphalt (HMA) Pavements (Base, Leveling or Surface Course)
- Item P-603, Bituminous Tack Coat
- MAG <sup>5</sup> 702, Crushed Aggregate Base
- MAG <sup>5</sup> 710, Asphalt Concrete

Item P-152 should specify the use of a modified proctor (ASTM D 1557) and 95 percent relative compaction. Measurement of in-place density should reference ASTM D6938 using Procedure A with direct transmission and a frequency of at least one test per 1,000 square yards. Proof-rolling after compaction should be specified using a 20-ton truck with tires inflated to at least 80 psi.

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<sup>5</sup> Asphalt concrete should conform to *Maricopa Association of Governments Uniform Standard Specifications for Public Works Construction (MAG)*, Current Edition.

Item P-209 should specify the use of a modified proctor (ASTM D 1557) and 100 percent relative compaction. Measurement of in-place density should reference ASTM D6938 using Procedure A with direct transmission and a frequency of at least one test per 1,000 square yards. Proof-rolling after compaction should be specified using a 20-ton truck with tires inflated to at least 80 psi.

Item P-304 should reference ASTM D6938 to evaluate relative compaction. Cement should conform to ASTM C150 Type I or II. A bond breaker should be required between P-304 and P-401 or P-403 to reduce the potential for reflective cracking.

Items P-401 and P-403 should specify the Marshall Design Criteria corresponding to 75 blows and Gradation 3 for the aggregate. It is our understanding that PG 76-22 and PG 70-22 bituminous material have been specified and used recently for other airports in Arizona and that, AC 150/5370-10G recommends against the use of binders with a rating above a -22 on the low end. Therefore, PG 76-22 or PG 70-22 should be specified.

Item P-403 is similar to P-401, but should specify its use as a base layer.

MAG 710 Asphalt Concrete should be specified as 1/2-inch, Low Traffic.

## 7.6 Compliance

Recommendations for pavements supported on compacted fills or prepared subgrade depend upon compliance with the **EARTHWORK** recommendations. To assess compliance, observation and testing should be performed under the direction of a WT geotechnical engineer. Please contact us to provide these observation and testing services.

## 8.0 ADDITIONAL SERVICES

The recommendations provided in this report are based on the assumption that a sufficient schedule of tests and observations will be performed during construction to verify compliance. At a minimum, these tests and observations should be comprised of the following:

- Observations and testing during site preparation and earthwork,
- Observation of foundation excavations, and
- Consultation as may be required during construction.

Retaining the geotechnical engineer who developed your report to provide construction observation is the best way to verify compliance and to help you manage the risks associated with unanticipated conditions.

## 9.0 LIMITATIONS

This report has been prepared assuming the project criteria described in **2.0 PROJECT DESCRIPTION**. If changes in the project criteria occur, or if different subsurface conditions are encountered or become known, the conclusions and recommendations presented herein shall become invalid. In any such event, WT should be contacted in order to assess the effect that such variations may have on our conclusions and recommendations. If WT is not retained for the construction observation and testing services to determine compliance with this report, our professional responsibility is accordingly limited.

The recommendations presented are based entirely upon data derived from a limited number of samples obtained from widely spaced explorations. The attached logs are indicators of subsurface conditions only at the specific locations and times noted. This report assumes the uniformity of the geology and soil structure between explorations, however variations can and often do exist. Whenever any deviation, difference, or change is encountered or becomes known, WT should be contacted.

This report is for the exclusive benefit of our client alone. There are no intended third-party beneficiaries of our contract with the client or this report, and nothing contained in the contract or this report shall create any express or implied contractual or any other relationship with, or claim or cause of action for, any third party against WT.

This report is valid for the earlier of one year from the date of issuance, a change in circumstances, or discovered variations. After expiration, no person or entity shall rely on this report without the express written authorization of WT.

## **10.0 CLOSURE**

We prepared this report as an aid to the designers of the proposed project. The comments, statements, recommendations and conclusions set forth in this report reflect the opinions of the authors. These opinions are based upon data obtained at the location of the explorations, and from laboratory tests. Work on your project was performed in accordance with generally accepted standards and practices utilized by professionals providing similar services in this locality. No other warranty, express or implied, is made.



NOT TO SCALE: FOR REFERENCE ONLY

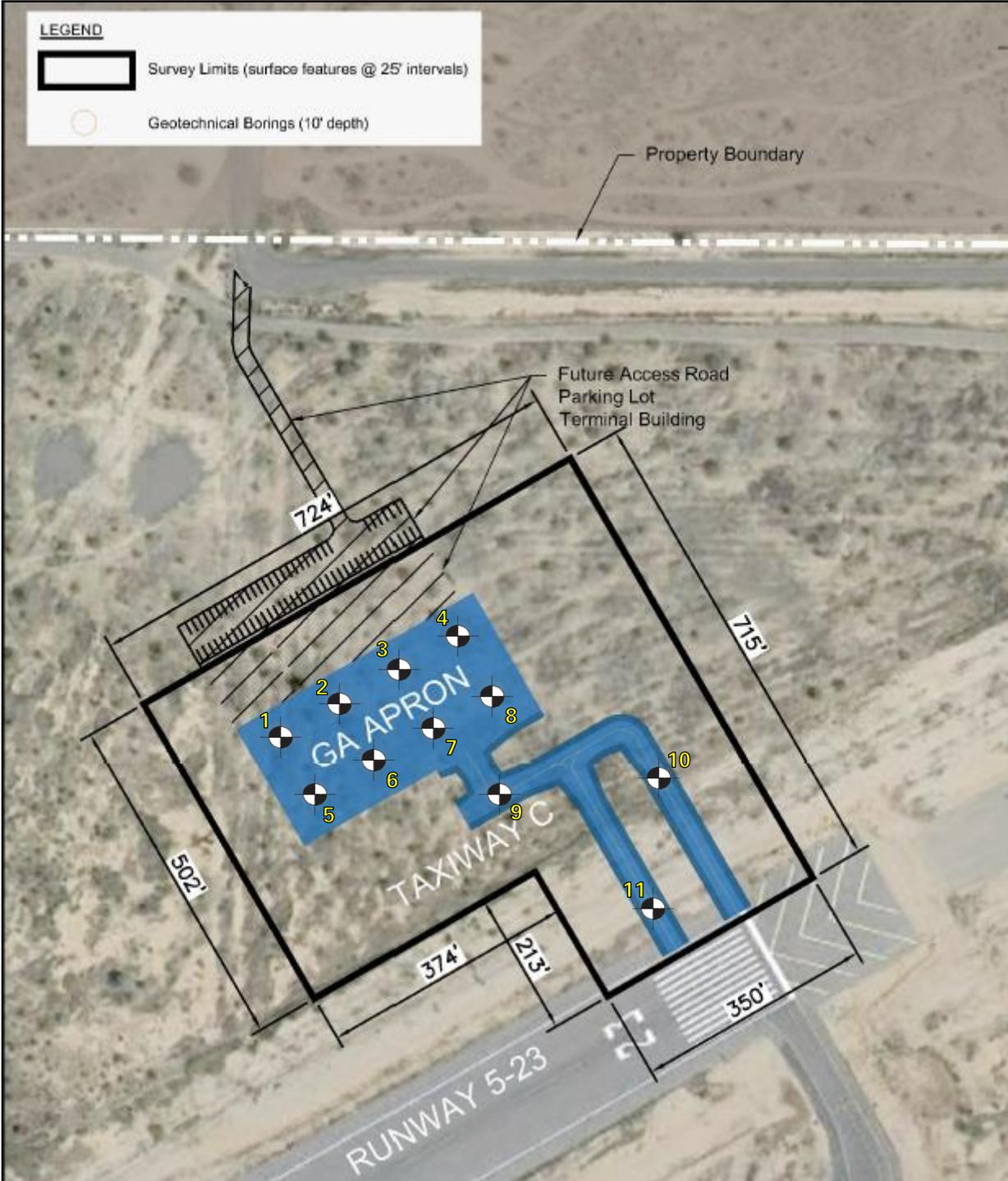


PROJECT: COOLIDGE MUNICIPAL AIRPORT  
GA APRON AND TAXIWAY C  
JOB NO.: 29-224083-0

PLATE

1

VICINITY MAP



**LEGEND**  
 [Black Outline] Survey Limits (surface features @ 25' intervals)  
 [Circle with Crosshair] Geotechnical Borings (10' depth)

**LEGEND**  
 [Circle with Crosshair] APPROXIMATE BORING LOCATION

NOT TO SCALE: FOR REFERENCE ONLY



PROJECT: COOLIDGE MUNICIPAL AIRPORT  
 GA APRON AND TAXIWAY C  
 JOB NO.: 29-224083-0

PLATE  
**2**

**BORING LOCATION DIAGRAM**

<b>Allowable Soil Bearing Capacity</b>	The recommended maximum contact stress developed at the interface of the foundation element and the supporting material.
<b>Backfill</b>	A specified material placed and compacted in a confined area.
<b>Base Course</b>	A layer of specified aggregate material placed on a subgrade or subbase.
<b>Base Course Grade</b>	Top of base course.
<b>Bench</b>	A horizontal surface in a sloped deposit.
<b>Caisson/Drilled Shaft</b>	A concrete foundation element cast in a circular excavation which may have an enlarged base (or belled caisson).
<b>Concrete Slabs-On-Grade</b>	A concrete surface layer cast directly upon base course, subbase or subgrade.
<b>Crushed Rock Base Course</b>	A base course composed of crushed rock of a specified gradation.
<b>Differential Settlement</b>	Unequal settlement between or within foundation elements of a structure.
<b>Engineered Fill</b>	Specified soil or aggregate material placed and compacted to specified density and/or moisture conditions under observations of a representative of a soil engineer.
<b>Existing Fill</b>	Materials deposited through the action of man prior to exploration of the site.
<b>Existing Grade</b>	The ground surface at the time of field exploration.
<b>Expansive Potential</b>	The potential of a soil to expand (increase in volume) due to absorption of moisture.
<b>Fill</b>	Materials deposited by the actions of man.
<b>Finished Grade</b>	The final grade created as a part of the project.
<b>Gravel Base Course</b>	A base course composed of naturally occurring gravel with a specified gradation.
<b>Heave</b>	Upward movement.
<b>Native Grade</b>	The naturally occurring ground surface.
<b>Native Soil</b>	Naturally occurring on-site soil.
<b>Rock</b>	A natural aggregate of mineral grains connected by strong and permanent cohesive forces. Usually requires drilling, wedging, blasting or other methods of extraordinary force for excavation.
<b>Sand and Gravel Base Course</b>	A base course of sand and gravel of a specified gradation.
<b>Sand Base Course</b>	A base course composed primarily of sand of a specified gradation.
<b>Scarify</b>	To mechanically loosen soil or break down existing soil structure.
<b>Settlement</b>	Downward movement.
<b>Soil</b>	Any unconsolidated material composed of discrete solid particles, derived from the physical and/or chemical disintegration of vegetable or mineral matter, which can be separated by gentle mechanical means such as agitation in water.
<b>Strip</b>	To remove from present location.
<b>Subbase</b>	A layer of specified material placed to form a layer between the subgrade and base course.
<b>Subbase Grade</b>	Top of subbase.
<b>Subgrade</b>	Prepared native soil surface.

**COARSE-GRAINED SOILS**  
LESS THAN 50% FINES

GROUP SYMBOLS	DESCRIPTION	MAJOR DIVISIONS
<b>GW</b>	WELL-GRADED GRAVEL OR WELL-GRADED GRAVEL WITH SAND, LESS THAN 5% FINES	GRAVELS MORE THAN HALF OF COARSE FRACTION IS LARGER THAN NO. 4 SIEVE SIZE
<b>GP</b>	POORLY-GRADED GRAVEL OR POORLY-GRADED GRAVEL WITH SAND, LESS THAN 5% FINES	
<b>GM</b>	SILTY GRAVEL OR SILTY GRAVEL WITH SAND, MORE THAN 12% FINES	
<b>GC</b>	CLAYEY GRAVEL OR CLAYEY GRAVEL WITH SAND, MORE THAN 12% FINES	
<b>SW</b>	WELL-GRADED SAND OR WELL-GRADED SAND WITH GRAVEL, LESS THAN 5% FINES	SANDS MORE THAN HALF OF COARSE FRACTION IS SMALLER THAN NO. 4 SIEVE SIZE
<b>SP</b>	POORLY-GRADED SAND OR POORLY-GRADED SAND WITH GRAVEL, LESS THAN 5% FINES	
<b>SM</b>	SILTY SAND OR SILTY SAND WITH GRAVEL, MORE THAN 12% FINES	
<b>SC</b>	CLAYEY SAND OR CLAYEY SAND WITH GRAVEL, MORE THAN 12% FINES	

**NOTE:** Coarse-grained soils receive dual symbols if they contain 5% to 12% fines (e.g., SW-SM, GP-GC).

**FINE-GRAINED SOILS**  
MORE THAN 50% FINES

GROUP SYMBOLS	DESCRIPTION	MAJOR DIVISIONS
<b>ML</b>	SILT, SILT WITH SAND OR GRAVEL, SANDY SILT, OR GRAVELLY SILT	SILTS AND CLAYS LIQUID LIMIT LESS THAN 50
<b>CL</b>	LEAN CLAY OF LOW TO MEDIUM PLASTICITY, SANDY CLAY, OR GRAVELLY CLAY	
<b>OL</b>	ORGANIC SILT OR ORGANIC CLAY OF LOW TO MEDIUM PLASTICITY	
<b>MH</b>	ELASTIC SILT, SANDY ELASTIC SILT, OR GRAVELLY ELASTIC SILT	SILTS AND CLAYS LIQUID LIMIT MORE THAN 50
<b>CH</b>	FAT CLAY OF HIGH PLASTICITY, SANDY FAT CLAY, OR GRAVELLY FAT CLAY	
<b>OH</b>	ORGANIC SILT OR ORGANIC CLAY OF HIGH PLASTICITY	
<b>PT</b>	PEAT AND OTHER HIGHLY ORGANIC SOILS	HIGHLY ORGANIC SOILS

**NOTE:** Fine-grained soils may receive dual classification based upon plasticity characteristics (e.g. CL-ML).

**SOIL SIZES**

COMPONENT	SIZE RANGE
BOULDERS	Above 12 in.
COBBLES	3 in. – 12 in.
GRAVEL	No. 4 – 3 in.
Coarse	¾ in. – 3 in.
Fine	No. 4 – ¾ in.
SAND	No. 200 – No. 4
Coarse	No. 10 – No. 4
Medium	No. 40 – No. 10
Fine	No. 200 – No. 40
Fines (Silt or Clay)	Below No. 200

**NOTE:** Only sizes smaller than three inches are used to classify soils

**CONSISTENCY**

CLAYS & SILTS	BLOWS PER FOOT
VERY SOFT	0 – 2
SOFT	3 – 4
FIRM	5 – 8
STIFF	9 – 15
VERY STIFF	16 – 30
HARD	OVER 30

**RELATIVE DENSITY**

SANDS & GRAVELS	BLOWS PER FOOT
VERY LOOSE	0 – 4
LOOSE	5 – 10
MEDIUM DENSE	11 – 30
DENSE	31 – 50
VERY DENSE	OVER 50

**NOTE:** Number of blows using 140-pound hammer falling 30 inches to drive a 2-inch-OD (1½-inch ID) split-barrel sampler (ASTM D1586).

**PLASTICITY OF FINE GRAINED SOILS**

PLASTICITY INDEX	TERM
0	NON-PLASTIC
1 – 7	LOW
8 – 20	MEDIUM
Over 20	HIGH

**DEFINITION OF WATER CONTENT**

DRY
SLIGHTLY DAMP
DAMP
MOIST
WET
SATURATED



**METHOD OF CLASSIFICATION**

PLATE

**A-2**

The number shown in "**BORING NO.**" refers to the approximate location of the same number indicated on the "Boring Location Diagram" as positioned in the field by pacing or measurement from property lines and/or existing features, or through the use of Global Positioning System (GPS) devices. The accuracy of GPS devices is somewhat variable.

"**DRILLING TYPE**" refers to the exploratory equipment used in the boring wherein **HSA = hollow stem auger**, and the dimension presented is the outside diameter of the HSA used.

"**N**" in "**BLOW COUNTS**" refers to a 2-inch outside diameter split-barrel sampler driven into the ground with a 140 pound drop-hammer dropped 30 inches repeatedly until a penetration of 18 inches is achieved or until refusal. The number of blows, or "blow count", of the hammer is recorded for each of three 6-inch increments totaling 18 inches. The number of blows required for advancing the sampler for the last 12 inches (2<sup>nd</sup> and 3<sup>rd</sup> increments) is defined as the Standard Penetration Test (SPT) "**N**"-Value. Refusal to penetration is considered more than 50 blows per 6 inches. (Ref. ASTM D1586).

"**R**" in "**BLOW COUNTS**" refers to a 3-inch outside diameter ring-lined split barrel sampler driven into the ground with a 140 pound drop-hammer dropped 30 inches repeatedly until a penetration of 12 inches is achieved or until refusal. The number of blows required to advance the sampler 12 inches is defined as the "**R**" blow count. The "**R**" blow count requires an engineered conversion to an equivalent SPT N-Value. Refusal to penetration is considered more than 50 blows per foot. (Ref. ASTM D3550).

"**CS**" in "**BLOWS/FT.**" refers to a 2½-in. outside diameter California style split-barrel sampler, lined with brass sleeves, driven into the ground with a 140-pound hammer dropped 30 inches repeatedly until a penetration of 18 inches is achieved or until refusal. The number of blows of the hammer is recorded for each of the three 6-inch increments totaling 18 inches. The number of blows required for advancing the sampler for the last 12 inches (2<sup>nd</sup> and 3<sup>rd</sup> increments) is defined as the "**CS**" blow count. The "**CS**" blow count requires an engineered conversion to an equivalent SPT N-Value. Refusal to penetration is considered more than 50 blows for a 6-inch increment. (Ref. ASTM D 3550)

"**SAMPLE TYPE**" refers to the form of sample recovery, in which **N** = Split-barrel sample, **R** = Ring-lined sample, "**CS**" = California style split-barrel sample, **G** = Grab sample, **B** = Bucket sample, **C** = Core sample (ex. diamond bit rock coring).

"**DRY DENSITY (LBS/CU FT)**" refers to the laboratory-determined dry density in pounds per cubic foot. The symbol "**NR**" indicates that no sample was recovered.

"**WATER (MOISTURE) CONTENT**" (% of Dry Wt.) refers to the laboratory-determined water content in percent using the standard test method ASTM D2216.

"**USCS**" refers to the "Unified Soil Classification System" Group Symbol for the soil type as defined by ASTM D2487 and D2488. The soils were classified visually in the field, and where appropriate, classifications were modified by visual examination of samples in the laboratory and/or by appropriate tests.

These notes and boring logs are intended for use in conjunction with the purposes of our services defined in the text. Boring log data should not be construed as part of the construction plans nor as defining construction conditions.

Boring logs depict our interpretations of subsurface conditions at the locations and on the date(s) noted. Variations in subsurface conditions and characteristics may occur between borings. Groundwater levels may fluctuate due to seasonal variations and other factors.

The stratification lines shown on the boring logs represent our interpretation of the approximate boundary between soil or rock types based upon visual field classification at the boring location. The transition between materials is approximate and may be more or less gradual than indicated.



## BORING LOG NOTES

PLATE

**A-3**

**Project: COOLIDGE MUNICIPAL  
AIRPORT GA APRON  
AND TAXIWAY C**  
Project Number: 29-224083-0

# BORING NO. 1



Date(s) Drilled <b>10/18/2024</b>	Logged By <b>T. DOMINGUEZ</b>	Checked By <b>J. HEINECKE</b>
Drilling Method <b>HSA</b>	Drill Bit Size/Type <b>7"</b>	Total Depth of Borehole <b>11 FT</b>
Drill Rig Type <b>CME-75</b>	Drilling Contractor <b>GSI</b>	Approximate Surface Elevation <b>NOT DETERMINED</b>
Groundwater Level and Date Measured <b>NOT ENCOUNTERED</b>	Sampling Method(s) <b>Bulk, Ring</b>	Hammer Data <b>140-LB AUTOHAMMER</b>
Borehole Backfill <b>AUGER CUTTINGS</b>	Location <b>SEE LOCATION DIAGRAM</b>	

DEPTH (FEET)	MOISTURE CONTENT	DRY DENSITY (LBS/CU FT)	SAMPLE TYPE	SAMPLE	BLOW COUNTS	USCS	GRAPHIC	SOIL DESCRIPTION	REMARKS AND OTHER TESTS
0	5.2	99	R		8	SC		Clayey SAND; light brown, loose, slightly damp	
			G						
5			R		56			becomes dense	
10			R		30 50/5"			becomes very dense	
15								BORING STOPPED AT 11 FEET	

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**Project: COOLIDGE MUNICIPAL  
AIRPORT GA APRON  
AND TAXIWAY C**  
Project Number: 29-224083-0

# BORING NO. 2



Date(s) Drilled <b>10/18/2024</b>	Logged By <b>T. DOMINGUEZ</b>	Checked By <b>J. HEINECKE</b>
Drilling Method <b>HSA</b>	Drill Bit Size/Type <b>7"</b>	Total Depth of Borehole <b>8.5 FT</b>
Drill Rig Type <b>CME-75</b>	Drilling Contractor <b>GSI</b>	Approximate Surface Elevation <b>NOT DETERMINED</b>
Groundwater Level and Date Measured <b>NOT ENCOUNTERED</b>	Sampling Method(s) <b>Bulk, Ring</b>	Hammer Data <b>140-LB AUTOHAMMER</b>
Borehole Backfill <b>AUGER CUTTINGS</b>	Location <b>SEE LOCATION DIAGRAM</b>	

DEPTH (FEET)	MOISTURE CONTENT	DRY DENSITY (LBS/CU FT)	SAMPLE TYPE	SAMPLE	BLOW COUNTS	USCS	GRAPHIC	SOIL DESCRIPTION	REMARKS AND OTHER TESTS
0						SC		Clayey SAND; light brown, medium dense, slightly damp	
5.5	95		R		44				
			R		36			becomes very dense	
					50/5"				
								BORING STOPPED AT 8.5 FEET	

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**Project: COOLIDGE MUNICIPAL  
AIRPORT GA APRON  
AND TAXIWAY C**  
Project Number: 29-224083-0

# BORING NO. 3



Date(s) Drilled <b>10/18/2024</b>	Logged By <b>T. DOMINGUEZ</b>	Checked By <b>J. HEINECKE</b>
Drilling Method <b>HSA</b>	Drill Bit Size/Type <b>7"</b>	Total Depth of Borehole <b>10 FT</b>
Drill Rig Type <b>CME-75</b>	Drilling Contractor <b>GSI</b>	Approximate Surface Elevation <b>NOT DETERMINED</b>
Groundwater Level and Date Measured <b>NOT ENCOUNTERED</b>	Sampling Method(s) <b>Bulk, Ring</b>	Hammer Data <b>140-LB AUTOHAMMER</b>
Borehole Backfill <b>AUGER CUTTINGS</b>	Location <b>SEE LOCATION DIAGRAM</b>	

DEPTH (FEET)	MOISTURE CONTENT	DRY DENSITY (LBS/CU FT)	SAMPLE TYPE	SAMPLE	BLOW COUNTS	USCS	GRAPHIC	SOIL DESCRIPTION	REMARKS AND OTHER TESTS
0			G			SC		Clayey SAND; light brown, medium dense, slightly damp	
6.6	101		R		33				
5.3	110		R		31 50/6"			becomes very dense	
			R		50			becomes dense	
10								BORING STOPPED AT 10 FEET	
15									

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**Project: COOLIDGE MUNICIPAL  
AIRPORT GA APRON  
AND TAXIWAY C**  
Project Number: 29-224083-0

# BORING NO. 4



Date(s) Drilled <b>10/18/2024</b>	Logged By <b>T. DOMINGUEZ</b>	Checked By <b>J. HEINECKE</b>
Drilling Method <b>HSA</b>	Drill Bit Size/Type <b>7"</b>	Total Depth of Borehole <b>11 FT</b>
Drill Rig Type <b>CME-75</b>	Drilling Contractor <b>GSI</b>	Approximate Surface Elevation <b>NOT DETERMINED</b>
Groundwater Level and Date Measured <b>NOT ENCOUNTERED</b>	Sampling Method(s) <b>Bulk, Ring</b>	Hammer Data <b>140-LB AUTOHAMMER</b>
Borehole Backfill <b>AUGER CUTTINGS</b>	Location <b>SEE LOCATION DIAGRAM</b>	

DEPTH (FEET)	MOISTURE CONTENT	DRY DENSITY (LBS/CU FT)	SAMPLE TYPE	SAMPLE	BLOW COUNTS	USCS	GRAPHIC	SOIL DESCRIPTION	REMARKS AND OTHER TESTS
0			R		11	SC		Clayey SAND; light brown, loose, slightly damp	
			G						
5	3.2	116	R		32			becomes medium dense	
10			R		20 50/5"			becomes very dense	
15								BORING STOPPED AT 11 FEET	

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**Project: COOLIDGE MUNICIPAL  
AIRPORT GA APRON  
AND TAXIWAY C**  
Project Number: 29-224083-0

# BORING NO. 5



Date(s) Drilled <b>10/18/2024</b>	Logged By <b>T. DOMINGUEZ</b>	Checked By <b>J. HEINECKE</b>
Drilling Method <b>HSA</b>	Drill Bit Size/Type <b>7"</b>	Total Depth of Borehole <b>8.5 FT</b>
Drill Rig Type <b>CME-75</b>	Drilling Contractor <b>GSI</b>	Approximate Surface Elevation <b>NOT DETERMINED</b>
Groundwater Level and Date Measured <b>NOT ENCOUNTERED</b>	Sampling Method(s) <b>Bulk, Ring</b>	Hammer Data <b>140-LB AUTOHAMMER</b>
Borehole Backfill <b>AUGER CUTTINGS</b>	Location <b>SEE LOCATION DIAGRAM</b>	

DEPTH (FEET)	MOISTURE CONTENT	DRY DENSITY (LBS/CU FT)	SAMPLE TYPE	SAMPLE	BLOW COUNTS	USCS	GRAPHIC	SOIL DESCRIPTION	REMARKS AND OTHER TESTS
0						SC		Clayey SAND; light brown, medium dense, slightly damp	
3.7	3.7	103	R		20				
5			R		68			becomes dense	
8.5								BORING STOPPED AT 8.5 FEET	

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**Project: COOLIDGE MUNICIPAL  
AIRPORT GA APRON  
AND TAXIWAY C**  
Project Number: 29-224083-0

# BORING NO. 6



Date(s) Drilled <b>10/18/2024</b>	Logged By <b>T. DOMINGUEZ</b>	Checked By <b>J. HEINECKE</b>
Drilling Method <b>HSA</b>	Drill Bit Size/Type <b>7"</b>	Total Depth of Borehole <b>10 FT</b>
Drill Rig Type <b>CME-75</b>	Drilling Contractor <b>GSI</b>	Approximate Surface Elevation <b>NOT DETERMINED</b>
Groundwater Level and Date Measured <b>NOT ENCOUNTERED</b>	Sampling Method(s) <b>Bulk, Ring</b>	Hammer Data <b>140-LB AUTOHAMMER</b>
Borehole Backfill <b>AUGER CUTTINGS</b>	Location <b>SEE LOCATION DIAGRAM</b>	

DEPTH (FEET)	MOISTURE CONTENT	DRY DENSITY (LBS/CU FT)	SAMPLE TYPE	SAMPLE	BLOW COUNTS	USCS	GRAPHIC	SOIL DESCRIPTION	REMARKS AND OTHER TESTS
0			G			SC		Clayey SAND; light brown, loose, slightly damp	
4.0	99		R		14				
5			R		11 50/4"			becomes very dense	
10			R		72			becomes dense	
10								BORING STOPPED AT 10 FEET	
15									

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**Project: COOLIDGE MUNICIPAL  
AIRPORT GA APRON  
AND TAXIWAY C**  
Project Number: 29-224083-0

# BORING NO. 7



Date(s) Drilled <b>10/18/2024</b>	Logged By <b>T. DOMINGUEZ</b>	Checked By <b>J. HEINECKE</b>
Drilling Method <b>HSA</b>	Drill Bit Size/Type <b>7"</b>	Total Depth of Borehole <b>11 FT</b>
Drill Rig Type <b>CME-75</b>	Drilling Contractor <b>GSI</b>	Approximate Surface Elevation <b>NOT DETERMINED</b>
Groundwater Level and Date Measured <b>NOT ENCOUNTERED</b>	Sampling Method(s) <b>Bulk, Ring</b>	Hammer Data <b>140-LB AUTOHAMMER</b>
Borehole Backfill <b>AUGER CUTTINGS</b>	Location <b>SEE LOCATION DIAGRAM</b>	

DEPTH (FEET)	MOISTURE CONTENT	DRY DENSITY (LBS/CU FT)	SAMPLE TYPE	SAMPLE	BLOW COUNTS	USCS	GRAPHIC	SOIL DESCRIPTION	REMARKS AND OTHER TESTS
0			R		14	SC		Clayey SAND; light brown, loose, slightly damp	
			G						
5	4.7	106	R		19			becomes medium dense	
10			R		37				
15								BORING STOPPED AT 11 FEET	

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**Project: COOLIDGE MUNICIPAL  
AIRPORT GA APRON  
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Project Number: 29-224083-0

# BORING NO. 8



Date(s) Drilled <b>10/18/2024</b>	Logged By <b>T. DOMINGUEZ</b>	Checked By <b>J. HEINECKE</b>
Drilling Method <b>HSA</b>	Drill Bit Size/Type <b>7"</b>	Total Depth of Borehole <b>8.5 FT</b>
Drill Rig Type <b>CME-75</b>	Drilling Contractor <b>GSI</b>	Approximate Surface Elevation <b>NOT DETERMINED</b>
Groundwater Level and Date Measured <b>NOT ENCOUNTERED</b>	Sampling Method(s) <b>Bulk, Ring</b>	Hammer Data <b>140-LB AUTOHAMMER</b>
Borehole Backfill <b>AUGER CUTTINGS</b>	Location <b>SEE LOCATION DIAGRAM</b>	

DEPTH (FEET)	MOISTURE CONTENT	DRY DENSITY (LBS/CU FT)	SAMPLE TYPE	SAMPLE	BLOW COUNTS	USCS	GRAPHIC	SOIL DESCRIPTION	REMARKS AND OTHER TESTS
0						SC		Clayey SAND; light brown, medium dense, slightly damp	
6.6	6.6	99	R		22				
34			R		34			becomes very dense	
50/5"					50/5"			BORING STOPPED AT 8.5 FEET	

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**Project: COOLIDGE MUNICIPAL  
AIRPORT GA APRON  
AND TAXIWAY C**  
Project Number: 29-224083-0

# BORING NO. 9



Date(s) Drilled <b>10/18/2024</b>	Logged By <b>T. DOMINGUEZ</b>	Checked By <b>J. HEINECKE</b>
Drilling Method <b>HSA</b>	Drill Bit Size/Type <b>7"</b>	Total Depth of Borehole <b>10 FT</b>
Drill Rig Type <b>CME-75</b>	Drilling Contractor <b>GSI</b>	Approximate Surface Elevation <b>NOT DETERMINED</b>
Groundwater Level and Date Measured <b>NOT ENCOUNTERED</b>	Sampling Method(s) <b>Bulk, Ring</b>	Hammer Data <b>140-LB AUTOHAMMER</b>
Borehole Backfill <b>AUGER CUTTINGS</b>	Location <b>SEE LOCATION DIAGRAM</b>	

DEPTH (FEET)	MOISTURE CONTENT	DRY DENSITY (LBS/CU FT)	SAMPLE TYPE	SAMPLE	BLOW COUNTS	USCS	GRAPHIC	SOIL DESCRIPTION	REMARKS AND OTHER TESTS
0			G			SC		Clayey SAND; light brown, medium dense, slightly damp	
5.0	96		R		25				
			R		44				
			R		26			becomes very dense	
10					50/5"			BORING STOPPED AT 10 FEET	
15									

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**Project: COOLIDGE MUNICIPAL  
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Project Number: 29-224083-0

# BORING NO. 10



Date(s) Drilled <b>10/18/2024</b>	Logged By <b>T. DOMINGUEZ</b>	Checked By <b>J. HEINECKE</b>
Drilling Method <b>HSA</b>	Drill Bit Size/Type <b>7"</b>	Total Depth of Borehole <b>11 FT</b>
Drill Rig Type <b>CME-75</b>	Drilling Contractor <b>GSI</b>	Approximate Surface Elevation <b>NOT DETERMINED</b>
Groundwater Level and Date Measured <b>NOT ENCOUNTERED</b>	Sampling Method(s) <b>Bulk, Ring</b>	Hammer Data <b>140-LB AUTOHAMMER</b>
Borehole Backfill <b>AUGER CUTTINGS</b>	Location <b>SEE LOCATION DIAGRAM</b>	

DEPTH (FEET)	MOISTURE CONTENT	DRY DENSITY (LBS/CU FT)	SAMPLE TYPE	SAMPLE	BLOW COUNTS	USCS	GRAPHIC	SOIL DESCRIPTION	REMARKS AND OTHER TESTS
0	2.0	102	R		13	SC		Clayey SAND; light brown, loose, slightly damp	
			G						
5			R		49			becomes dense	
10			R		48			becomes medium dense	
15								BORING STOPPED AT 11 FEET	

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**Project: COOLIDGE MUNICIPAL  
AIRPORT GA APRON  
AND TAXIWAY C**  
Project Number: 29-224083-0

# BORING NO. 11



Date(s) Drilled <b>10/18/2024</b>	Logged By <b>T. DOMINGUEZ</b>	Checked By <b>J. HEINECKE</b>
Drilling Method <b>HSA</b>	Drill Bit Size/Type <b>7"</b>	Total Depth of Borehole <b>8.5 FT</b>
Drill Rig Type <b>CME-75</b>	Drilling Contractor <b>GSI</b>	Approximate Surface Elevation <b>NOT DETERMINED</b>
Groundwater Level and Date Measured <b>NOT ENCOUNTERED</b>	Sampling Method(s) <b>Bulk, Ring</b>	Hammer Data <b>140-LB AUTOHAMMER</b>
Borehole Backfill <b>AUGER CUTTINGS</b>	Location <b>SEE LOCATION DIAGRAM</b>	

DEPTH (FEET)	MOISTURE CONTENT	DRY DENSITY (LBS/CU FT)	SAMPLE TYPE	SAMPLE	BLOW COUNTS	USCS	GRAPHIC	SOIL DESCRIPTION	REMARKS AND OTHER TESTS
0						SC		Clayey SAND; light brown, medium dense, slightly damp	
5.6	5.6	109	R		31				
			G						
			R		46				
								BORING STOPPED AT 8.5 FEET	

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Boring No.	Sample Depth (ft)	USCS Class.	Percent Passing #200	Atterberg Limits		Initial Dry Density (pcf)	Initial Water Content (%)	Compression Properties			Moisture-Density Relationship			Expansion Properties			Soluble Sulfates (ppm)	Soluble Chlorides (ppm)	Remarks
				LL	PI			Surcharge (ksf)	Total Compression (%)		Maximum Dry Density (pcf)	Optimum Moisture Content (%)	Method	Surcharge (ksf)	Expansion (%)	Expansion Index (EI)			
									In-Situ	After Saturation									
1,3,6,8	0-3	SC	37	26	10	116.3	9.0				128.3	8.0	C	0.1	0.7			1,2,12,13	
9,10,11	0-3	SC	36	23	8	116.0	9.2				133.9	7.1	C	0.1	0.8			1,2,12,13	
1	0-1	SC				99	5.2											11	
2	0-3	SC														8	37	6,7,11,15	
2	2.5-3.5	SC				95	5.5											11	
3	1-2	SC				101	6.6	0.5	1.3									11	
								1.0	2.2	4.2								2	
								1.5		5.4								2	
								2.5		7.6								2	
3	6-7	SC				110	5.3											11	
4	5-6	SC				116	3.2											11	
5	2.5-3.5	SC				103	3.7											11	
6	1-2	SC				99	4.0	0.5	0.9									11	
								1.0	1.8	9.1								2	
								1.5		10.6								2	
								2.5		12.2								2	
7	5-6	SC				106	4.7											11	
8	2.5-3.5	SC				99	6.6											11	
9	1-2	SC				96	5.0											11	
10	0-1	SC				102	2.0											11	
11	2.5-3.5	SC				109	5.6											11	

**Remarks**

1. Compacted density is approximately 95% of ASTM D698 maximum density at a moisture content slightly below optimum.  
2. Submerged to approximate saturation.  
3. Slight rebound after saturation.  
4. Sample disturbance observed.  
5. Expansion Index (EI) test in accordance with ASTM D4829.

6. Chloride (ARIZ 736a) by Motzz Laboratory Inc.  
7. Sulfate (ARIZ 733a) by Motzz Laboratory Inc.  
8. pH (ARIZ 237b).  
9. Minimum Resistivity (ARIZ 236c).  
10. Test Method ASTM D698 / AASHTO T99.  
11. Field Visual Classification (ASTM D 2488).

12. Laboratory Soil Classification (ASTM D 2487).  
13. Test Method ASTM D1557 / AASHTO T180.  
14. From the ADOT Family of Curves for Maricopa County.  
15. See Corrosion Plate.  
16. Initial Dry Density and Initial Water Content from Remolded Swell.  
**Notes:** Initial Dry Density and Initial Water Content are in-situ values unless otherwise noted.  
NP = Non-Plastic    NV = No Value

	PROJECT: <b>COOLIDGE MUNICIPAL AIRPORT GA APRON AND TAXIWAY C</b>	<b>PLATE</b>
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<b>SOIL PROPERTIES</b>		<b>B-1</b>

Boring No.	Sample Depth (ft)	USCS Class.	Soluble		pH	Minimum Resistivity (Ohm-Cm)	Remarks
			Sulfate (ppm)	Chloride (ppm)			
2	0-3	SC	8	37	7.5	2055	1,2,3,4,5

**REMARKS**

1. Chloride Content (ARIZ 736a).
2. Sulfate Content (ARIZ 733a).
3. pH (ARIZ 237b).
4. Minimum Resistivity (ARIZ 236c).
5. Field Visual Classification (ASTM D2488).
6. Laboratory Soil Classification (ASTM D 2487).

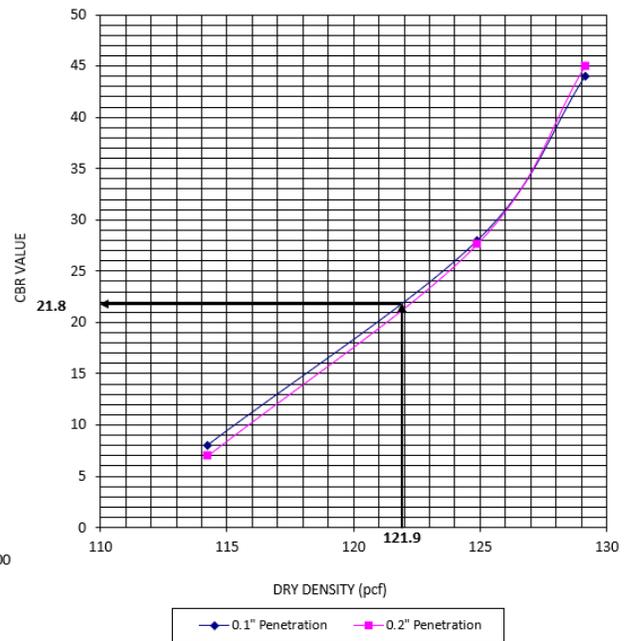
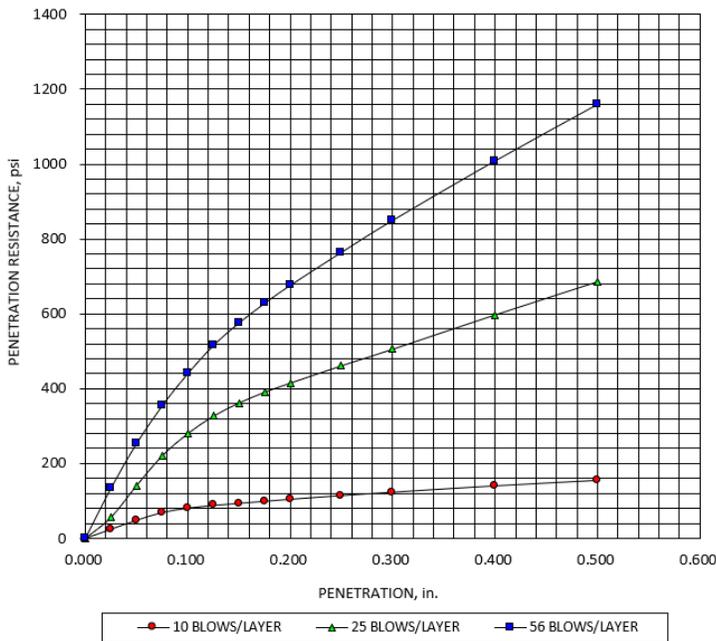
**NOTES:** Initial Dry Density and Initial Water Content are in-situ values unless otherwise noted. Sulfate and Chloride contents by Motzz Laboratory Inc.

NP = Non-plastic    NV = No Value

 <p><b>Western Technologies</b> An RMA Company</p>	PROJECT: <b>COOLIDGE MUNIPAL AIRPORT</b> JOB NO.: <b>29-224083-0</b>	<b>PLATE</b>  <b>B-2</b>
	<b>SOIL PROPERTIES</b>	

## CALIFORNIA BEARING RATIO (CBR) Borings 1-8<sup>1</sup> (0'-3')

Compacted Specimen Results	Procedure: <span style="color: blue;">ASTM D1883</span>		
Compactive Effort (Blows per Layer):	10	30	60
Dry Density at Compaction, pcf:	114.2	124.9	129.2
Percent of Maximum Dry Density:	89.0	97.3	100.7
Percent Moisture Before Compaction:	8.0	8.5	8.2
Percent Moisture After Compaction:	8.7	9.4	8.8
Percent Moisture after Soaking (Avg. of Total Sample):	14.8	11.4	10.4
Dry Density after Soaking, pcf:	114.0	124.9	128.8
Percent Moisture after Soaking (Top 1 in.):	16.5	12.9	11.8
Swell, %:	0.2	0.1	0.1
Corrected CBR at 0.100 in. Penetration:	8	28	44
Corrected CBR at 0.200 in. Penetration:	7	28	45
Surcharge Weight, lbs.:	10	10	10
California Bearing Ratio (CBR) at 95% Relative Compaction:	<b>22</b>		



<sup>1</sup> Tested sample is composite material of boreholes 1, 3, 6, and 8.



PROJECT: **COOLIDGE MUNICIPAL AIRPORT  
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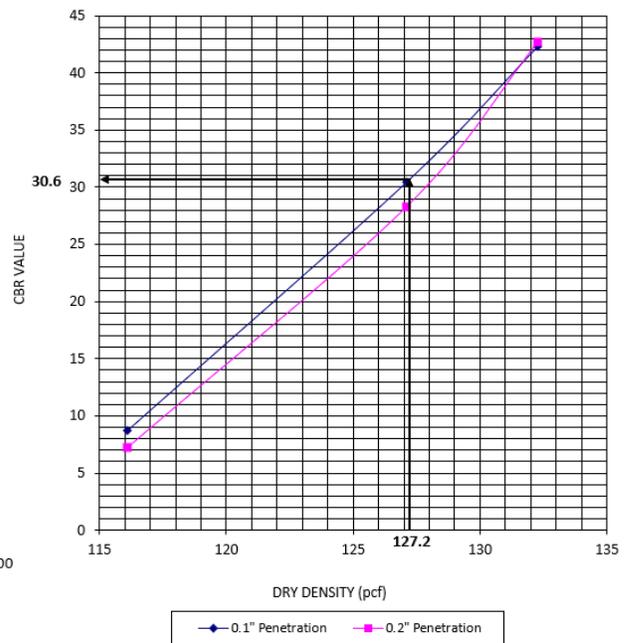
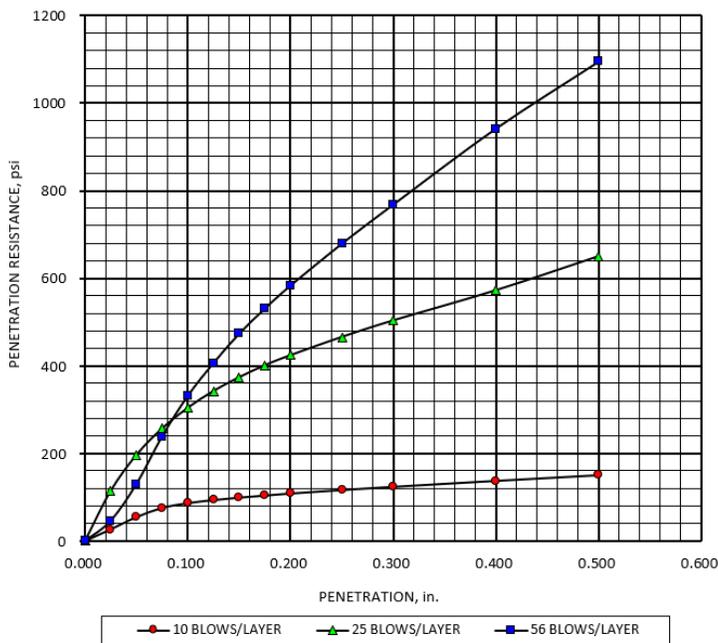
PLATE

**B-3**

**CALIFORNIA BEARING RATIO**

## CALIFORNIA BEARING RATIO (CBR) Borings 9-11<sup>1</sup> (0'-3')

Compacted Specimen Results	Procedure: <span style="color: blue;">ASTM D1883</span>		
Compactive Effort (Blows per Layer):	10	30	60
Dry Density at Compaction, pcf:	116.1	127.1	132.3
Percent of Maximum Dry Density:	86.7	94.9	98.8
Percent Moisture Before Compaction:	7.6	7.5	7.4
Percent Moisture After Compaction:	7.9	7.6	7.6
Percent Moisture after Soaking (Avg. of Total Sample):	13.7	10.5	9.4
Dry Density after Soaking, pcf:	116.1	127.1	132.0
Percent Moisture after Soaking (Top 1 in.):	14.4	11.5	11.6
Swell, %:	0.1	0.1	0.2
Corrected CBR at 0.100 in. Penetration:	9	30	42
Corrected CBR at 0.200 in. Penetration:	7	28	43
Surcharge Weight, lbs.:	10	10	10
California Bearing Ratio (CBR) at 95% Relative Compaction:	<b>31</b>		



<sup>1</sup> Tested sample is composite material of boreholes 9, 10, and 11.



PROJECT: **COOLIDGE MUNICIPAL AIRPORT  
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PLATE

**B-4**

**CALIFORNIA BEARING RATIO**